

GENERAL NOTES:

1.

READ STRUCTURAL DRAWINGS IN CONJUNCTION WITH THE REMAINDER OF THE CONTRACT DRAWINGS AND DOCUMENTS.
2.

VERIFY ALL DIMENSIONS ON THE STRUCTURAL DRAWINGS WITH THE REMAINDER OF THE CONTRACT DRAWINGS BEFORE CONSTRUCTION. ANY DISCREPANCIES OR ERRORS MUST BE REPORTED TO THE ENGINEER PRIOR TO STARTING THE WORK.
3.

THE CONTRACTOR IS TO VERIFY ALL DIMENSIONS PRIOR TO COMMENCEMENT OF WORK. DO NOT SCALE FROM DRAWINGS. REPORT ANY DISCREPANCIES TO THE CONSULTANT FOR CLARIFICATION BEFORE PROCEEDING.
4.

DESIGN LOADS INDICATED ARE UNFACTORED UNLESS NOTED OTHERWISE.
5.

DESIGN LIVE LOADS FOR EACH PORTION OF THE STRUCTURE ARE SHOWN. DO NOT EXCEED THESE LOADS DURING CONSTRUCTION.
6.

STRUCTURAL DESIGN IS BASED ON THE LATEST EDITION OF THE NATIONAL AND ONTARIO BUILDING CODES, SUBSTRUCTURES AND WATER RETAINING TANKS, RESERVOIRS AND CONDUITS HAVE BEEN DESIGNED IN ACCORDANCE WITH CODE REQUIREMENTS FOR ENVIRONMENTAL CONCRETE STRUCTURES (ACI) 350-06 EXCEPT WHERE IT WAS NOT CONSIDERED APPLICABLE.
7.

FEATURES OF CONSTRUCTION NOT FULLY SHOWN ARE OF THE SAME CHARACTER AS THOSE NOTED FOR SIMILAR CONDITIONS.
8.

COORDINATE PLACEMENT AND LOCATION OF ITEMS BY SUBSEQUENT TRADES. RELEVANT TRADES SHALL REVIEW PRIOR TO ERECTION AND/OR INSTALLATION.
9.

REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR SMALL OPENINGS, SLEEVES, RECESSES, DEPRESSIONS, SUMPS, TRENCHES, CURBS, HOUSEKEEPING PADS, EQUIPMENT BASES, AND SLOPES NOT INDICATED ON THE STRUCTURAL DRAWINGS.
10.

DO NOT SCALE DRAWINGS.
11.

ALL CODES REFERENCED ARE TO BE THE LATEST VERSION AT THE DATE OF ISSUE.
12.

SUBMIT DRAWINGS OF EXISTING BUILDING SHORING FOR REVIEW. DRAWINGS TO BE SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.

FOUNDATION

1.

THE DESIGN OF TEMPORARY WORKS IS THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTROL OF THE GROUND WATER SHALL BE CARRIED OUT BY A SPECIALIZED FOUNDATION CONSULTANT, ENGAGED BY THE CONTRACTOR. THE COSTS OF ANY ADDITION GEOTECHNICAL INVESTIGATION AND/OR TESTING IS INCIDENTAL TO THE WORK AND WILL NOT BE CONSIDERED AS EXTRA COST TO THE OWNER.
2.

THE SPECIALIZED FOUNDATION CONSULTANTS SHALL BE RESPONSIBLE FOR THE DESIGN, INSTALLATION, TESTING, MONITORING AND, IF REQUIRED, REMOVAL OF TEMPORARY SHORING AND DEWATERING SYSTEMS.
3.

BACKFILL AGAINST FOUNDATION WALL IN SUCH A MANNER THAT THE LEVEL OFBACKFILLING ON ONE SIDE OF THE WALL IS NEVER MORE THAN 450mm DIFFERENCE FROM THE LEVEL ON THE OTHER SIDE OF THE WALL UNLESS TEMPORARY SUPPORT FOR THE WALL IS PROVIDED.
4.

SOFT AREAS UNCOVERED ON EXCAVATION SHALL BE SUB EXCAVATED TO SOUND MATERIAL AND FILLED WITH GRANULAR 'A' SOIL COMPACTED TO 100% STANDARD PROCTOR DRY DENSITY.
5.

PLACE SLAB ON GRADE ON SOIL CAPABLE OF SUSTAINING 24 kPa WITHOUT SETTLEMENT RELATIVE TO THE BUILDING FOOTINGS.

STRUCTURAL STEEL:

1.

COORDINATE WITH MECHANICAL ENGINEER, ELECTRICAL ENGINEER AND ALL SUB-TRADES WHOSE WORK AFFECTS THE DETAILING, FABRICATION AND ERECTION OF THE STRUCTURAL STEEL. DO NOT CUT OPENINGS IN STRUCTURAL STEEL MEMBERS WITHOUT APPROVAL OF ENGINEER.
2.

PREPARE AND SUBMIT FULLY DETAILED AND DIMENSIONED DRAWINGS AND ERECTION DIAGRAMs. COPIES SHALL BE PROVIDED TO THE ENGINEER FOR REVIEW. ERECTION DRAWINGS SHALL BE SEALED BY THE FABRICATOR'S ENGINEER.
3.

COPIES OF THE ERECTION AND FABRICATION DRAWINGS SHALL BE PROVIDED AS REQUIRED FOR THE OWNER'S FILE AND/OR THE MUNICIPALITY.
4.

BOLTED CONNECTIONS SHALL BE MADE USING HIGH TENSILE STRENGTH BOLTS.
5.

PROVIDE TEMPORARY BRACING NECESSARY TO KEEP THE STRUCTURE TRUE AND PLUMB DURING CONSTRUCTION UNTIL PERMANENT BRACING IS INSTALLED. BRACING DESIGN SHALL BE PROVIDED AND CERTIFIED BY FABRICATOR'S ENGINEER.
6.

ALL CONNECTIONS TO BE STANDARD FRAME BEAM CONNECTIONS AND ARE TO BE DESIGNED AND CERTIFIED BY THE FABRICATOR'S ENGINEER AS PER CSA S16.1.
7.

VARIATIONS FROM PLUMB AND LEVEL:
EXTERIOR COLUMNS, SPANDREL BEAMS, AND ANGLES: ± 3/3000
OTHER MEMBERS: ± 6/3000
8.

STEEL SHALL BE THOROUGHLY CLEANED AND BE GIVEN ONE SHOP COAT OF ANTI-CORROSIVE PRIMER. AREAS AFFECTED BY WEATHERING, OR ANY OTHER DAMAGE SHALL HAVE THE RUST REMOVED AND BE "TOUCHED UP" IN THE FIELD. STEEL PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED UNLESS NOTED ELSEWHERE. GALVANIZING SHALL BE REMOVED LOCALLY PRIOR TO ANY STRUCTURAL FIELD WELDING.

MATERIALS:

1.

CONCRETE SLAB INFILL AND THICKENING SHALL BE 25 MPa, CLASS N.
CONCRETE PILE CAP AND FOUNDATION WALLS SHALL BE 35 MPa, CLASS N
2.

ALL REINFORCING BAR SHALL BE GRADE 400MPa, DEFORMED, CAN/CSA-G30.18.
3.

CONCRETE BLOCK SHALL CONFORM TO THE LATEST EDITION OF THE RELEVANT CODES AND STANDARDS AND THE BLOCK STRENGTH SHALL BE 15MPa ON NET AREA.
4.

MORTAR SHALL BE TYPE "S" UNLESS NOTED OTHERWISE.
5.

CONCRETE FILL IN REINFORCED MASONRY SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 20MPa.
6.

COLUMN BEARING GROUT SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 40MPa.
7.

STRUCTURAL STEEL TO CONFORM TO CAN/CSA-G40.21, UNO.

W SECTIONS: GRADE 350W
L AND C SECTIONS: GRADE 350W
HSS SECTIONS: GRADE 350 CLASS 'H'
8.

THE WELDING ELECTRODES SHALL CONFORM TO THE REQUIREMENTS OF CSA STANDARD W48 (LATEST EDITION).

MASONRY:

1.

ALL MASONRY CONSTRUCTION SHALL CONFORM TO THE RELEVANT CODES AND STANDARD AS REQUIRED IN THE PROJECT SPECIFICATION.
2.

THE MASONRY WORK HAS BEEN DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE APPLICABLE CODES AND STANDARDS AS REQUIRED BY THE PROJECT SPECIFICATIONS.
3.

NO MASONRY WORK SHALL BE PERMITTED WITH THE TEMPERATURE BELOW 5°C UNLESS PROVISIONS ARE MADE FOR HEATING THE MATERIALS AND WORK AREA AND PROTECTING THE WORK.
4.

OBTAIN ENGINEER'S PERMISSION FOR ALL OPENINGS, SLEEVES AND SLOTS OTHER THAN SHOWN ON THE STRUCTURAL DRAWINGS. WHERE DOWELS, ANCHOR BOLTS, ETC., ARE SHOWN PROJECTING INTO MASONRY, BUILD THESE INTO MASONRY VOIDS WITH GROUT.
5.

ALL MASONRY SHALL BE SET WITH FULLY FILLED JOINTS.
6.

CELLS TO BE REINFORCED SHALL BE KEPT CLEAN OF MORTAR.
7.

PROVIDE A MINIMUM 25mm GROUT UNDER ALL WALL PLATES AND BASE PLATES AND BEAR ON SOLID MASONRY OF 400mm (MINIMUM) DEPTH.
8.

PROVIDE AND INSTALL LINTELS OVER ALL OPENINGS OR RECESSES IN MASONRY WALLS INCLUDING THOSE FOR MECHANICAL OR ELECTRICAL SERVICES OR EQUIPMENT, IN ACCORDANCE WITH THE REQUIREMENTS OF THE LINTEL SCHEDULE.
9.

PROVIDE A MINIMUM LENGTH OF 200mm OF 100% SOLID MASONRY UNITS FOR BEARING OF STEEL, CONCRETE OR REINFORCED MASONRY LINTELS. FILL LINTELS WITH 20MPa CONCRETE GROUT CONTAINING 10mm AGGREGATE.
10.

ALL MASONRY WALLS TO BE REINFORCED WITH 15M @ 1200 UNLESS NOTED OTHERWISE.
11.

THE CONCRETE CONTRACTOR MUST PROVIDE REINFORCING DOWELS PROJECTING FROM CAST-IN-PLACE CONCRETE INTO BLOCK WALLS TO MATCH VERTICAL REINFORCING IN BLOCK WALLS. LAPS IN REINFORCING:

WIRE REINFORCING 150
10M 600
15M 750
20M 950
25M 1100
30M 1600
35M MECHANICAL SPLICE

12.

FILL CELLS CONTAINING VERTICAL REINFORCING WITH 20MPa CONCRETE GROUT CONTAINING 10mm AGGREGATE AND UP TO 250mm SLUMP, VIBRATE OR PUDDLE TO FILL CELLS COMPLETELY. USING JOINT MORTAR FOR FILLING THE CELLS IS NOT ACCEPTABLE AND WILL REQUIRE RECONSTRUCTION OF WALL.
13.

FILL CELLS IN 1500mm HIGH LIFTS OR IF CLEANOUTS ARE PROVIDED IN 2400mm HIGH LIFTS.
14.

PROVIDE CONTINUOUS LADDER TYPE JOINT REINFORCING AT 400mm c/c AND USE "CORNER-LOK" AT ALL WALL INTERSECTIONS. REINFORCING TO BE GALVANIZED TO ASTM A153 CLASS B2 (458g/m²).

FOR CAVITY WALL AND SINGLE WYTHE 3.65mm Ø WIRES (9 GAUGE)
FOR COMPOSITE WYTHE 4.76mm Ø WIRES
15.

ALL METAL ANCHORS TO SECURE WALLS EXPOSED TO WEATHER SHALL BE HOT DIP GALVANIZED.
16.

ANCHOR MASONRY TO STEEL COLUMNS WITH STEEL STRAP PLATE AS INDICATED.
17.

JOINTS IN MASONRY SHALL BE LOCATED AT LEAST 300mm FROM ANY OPENING IN THE WALL, UNLESS SPECIFICALLY NOTED OTHERWISE.

CAST-IN-PLACE CONCRETE:

1.

DO CONCRETE WORK IN ACCORDANCE WITH THE LATEST VERSION OF THE APPLICABLE CODES AND STANDARDS.
2.

SUBMIT REINFORCING DIAGRAMs BEFORE FABRICATION FOR REVIEW BY THE ENGINEER.
3.

REINFORCING IS TO BE GENERALLY DETAILED IN ACCORDANCE WITH RSIC, MANUAL OF STANDARD PRACTICE (LATEST EDITION). SPLICES SHALL CONFORM TO "TABLE OF CLASS 'B' TENSION LAPS AND STANDARD 90° HOOKS" PROVIDED ON THE DRAWINGS.
4.

THE CLEAR DISTANCE BETWEEN REINFORCING STEEL AND SURFACE OF CONCRETE SHALL BE AS FOLLOWS:

FORMED CONCRETE NOT EXPOSED TO WATER OR WEATHER: 40mm
FORMED CONCRETE EXPOSED TO WATER, WEATHER OR EARTH: 50mm
CONCRETE PLACED AGAINST MUD MAT: 50mm
CONCRETE PLACED AGAINST EARTH: 75mm
5.

UNLESS INDICATED OTHERWISE, ALL DOWELS SHALL HAVE THE SAME SIZE AND SPACING AS THE REINFORCING STEEL TO WHICH THEY ARE SPLICED.
6.

ALL REINFORCING STEEL PLACEMENT TO BE INSPECTED BY THE ENGINEER BEFORE PLACING THE CONCRETE.
7.

NO WELDING OF REINFORCING BARS SHALL BE PERMITTED, UNLESS APPROVAL IS OBTAINED FROM THE ENGINEER PRIOR TO CONSTRUCTION.
8.

DO NOT SAWCUT THE SLAB ON GRADE OR OTHER FLOORS, UNLESS SPECIFICALLY SHOWN AND DETAILED ON THE DESIGN DRAWINGS.
9.

ALL REINFORCING BARS SHALL BE SUPPORTED IN THE FORMS AND SPACED WITH STANDARD ACCESSORIES SO THAT THERE IS NO MOVEMENT DURING CONCRETE PLACEMENT.
10.

PROVIDE REINFORCING DOWELS PROJECTING FROM CAST-IN-PLACE CONCRETE INTO BLOCK WALLS TO MATCH VERTICAL REINFORCING IN BLOCK WALLS.

DELEGATED DESIGN:

1.

SUBMIT SHOP DRAWINGS FOR COMPONENTS REQUIRING DELEGATED DESIGN UNDER THE SEAL AND SIGNATURE OF THE ENGINEER RESPONSIBLE FOR THE DESIGN TO THE PRIME CONSULTANT FOR COORDINATION REVIEW.
2.

SHOP DRAWINGS SHALL CLEARLY INDICATE ANY LOADS IMPARTED TO THE STRUCTURE.
3.

CONNECTIONS TO THE PRIMARY STRUCTURE SHALL ACCOMMODATE DEFLECTIONS IN THE PRIMARY STRUCTURE. THE CONTRACTOR MAY SEEK GUIDANCE FROM THE ENGINEER OF RECORD AS TO DESIGN LOADS, DEFLECTIONS, ETC.

EXISTING STRUCTURES:

1.

DIMENSIONS PERTAINING TO EXISTING STRUCTURES SHALL BE VERIFIED IN THE FIELD BY THE CONTRACTOR, PRIOR TO THE COMMENCEMENT OF CONSTRUCTION AND FABRICATION. CIMA IS TO BE NOTIFIED OF ANY DISCREPANCIES.
2.

THE CONTRACTOR SHALL, AT THEIR OWN EXPENSE, REPAIR ANY DAMAGE TO THE EXISTING STRUCTURE, EQUIPMENT & FINISHES CAUSED BY THE CONSTRUCTION ACTIVITIES. REPAIRS SHALL BE SUBJECT TO THE ARCHITECT'S APPROVAL.

SHORING

1.

THE SITE CONDITIONS WILL NOT PERMIT AN OPEN EXCAVATION.
2.

ALL SIDES OF THE EXCAVATION MUST BE BRACED OR SHORED TO MAINTAIN STRICT ADHERENCE TO THE OCCUPATIONAL HEALTH AND SAFETY ACT.
3.

THE LINE OF THE SHORING SHOWN ON THE DRAWING HAS BEEN PROVIDED AS A GUIDE INDICATING THE GENERAL EXTENT AND LOCATION OF THE SYSTEM. HOWEVER, THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF A SYSTEM TO SUIT THEIR CONSTRUCTION REQUIREMENTS.
4.

THE CONTRACTOR SHALL ENGAGE A STRUCTURAL ENGINEER TO DESIGN AND INSPECT THE SHORING SYSTEM.
5.

THE CONTRACTOR SHALL ENGAGE A GEOTECHNICAL ENGINEER TO REVIEW AND CARRY OUT AN ASSESSMENT OF THE SITE CONDITIONS. BASED ON THIS WORK AND A REVIEW OF THE AVAILABLE GEOTECHNICAL INFORMATION, THE GEOTECHNICAL ENGINEER SHALL PROVIDE A DESIGN INFORMATION TO THE STRUCTURAL ENGINEER ENGAGED BY THE CONTRACTOR TO DESIGN THE SHORING SYSTEM.
6.

THE STRUCTURAL ENGINEER SHALL SUBMIT SEALED COPIES OF THE DESIGN DRAWINGS FOR REVIEW BY THE OWNER'S ENGINEER BEFORE PROCEEDING WITH THE WORK.
7.

THE CONTRACTOR SHALL NOT DISTURB OR DAMAGE ADJACENT STRUCTURES DURING EXCAVATION CONSTRUCTION, BACKFILL OR COMPACTION OPERATIONS, INCLUDING THE DRIVING OR REMOVAL OF ANY SHORING.
8.

THE CONTRACTOR SHALL PROVIDE FOR CONDUCTING PRECONDITION SURVEYS OF EXISTING ADJACENT BUILDINGS AND FOR DESIGNING APPROPRIATE CONSTRUCTION METHODS TO AVOID DAMAGE TO THESE BUILDINGS AND/OR PREMISES.

CLIMATE DATA FOR PICKERING				
IMPORTANCE CATEGORY: HIGH				
LOAD TYPE		LOAD		IMPORTANCE FACTOR
SNOW	GROUND SNOW LOAD (1/50 YEARS)	Ss	1.0 kPa	1.5
	ASSOCIATED RAIN LOAD (1/50 YEARS)	Sr	0.4 kPa	
WIND	REFERENCE VELOCITY WIND PRESSURE (1/50 YEARS)	q	0.48 kPa	1.5
EARTHQUAKE	PEAK GROUND ACCELERATION	PGA (E)	0.251	1.5
	HORIZONTAL SPECTRAL ACCELERATION VALUES (2% PROBABILITY OF EXCEEDANCE IN 50 YEARS)	Sa(0.2,E)	0.402	
		Sa(0.5,E)	0.222	
		Sa(1.0,E)	0.105	
		Sa(2.0,E)	0.022	
		Sa(5.0,E)	0.028	
		Sa(10.0,E)	0.008	
		SITE CLASS (ASSUMED)	E	

ABBREVIATIONS

ADDL	ADDITIONAL	L	LENGTH OF REBAR
AGGR	AGGREGATE	LG	LONG
ALT	ALTERNATE	LL	LOWER LEVEL or LIVE LOAD
ALUM	ALUMINUM	LLH	LONG LEG HORIZONTAL
APPROX	APPROXIMATE	LLV	LONG LEG VERTICAL
ARCH	ARCHITECTURAL	LNTL	LINTEL
B/	BOTTOM OF	LP	LOW POINT
BLDG	BUILDING	LW	LONG WAY
BM	BEAM	MAX	MAXIMUM
BPL	BASE PLATE	MC	MOMENT CONNECTION
BLL	BOTTOM LOWER LAYER	MECH	MECHANICAL
BOT	BOTTOM	MEMB	MEMBRANE
BRG	BEARING	MEZZ	MEZZANINE
BR	BRACING	MID	MIDDLE
BRDG	BRIDGING	MIN	MINIMUM
BS	BOTH SIDES	MIRR	MIRROR
BTWN	BETWEEN	MISC	MISCELLANEOUS
BUL	BOTTOM UPPER LAYER	NA	NOT APPLICABLE
BW	BOTH WAYS	NTS	NOT TO SCALE
C/C	CENTER TO CENTER	OC	ON CENTER
C/W	COMPLETE WITH	OD	OUTSIDE DIAMETER
CANT	CANTILEVER	OF	OUTSIDE FACE
CIP	CAST-IN-PLACE	OH	OVERHEAD
± or CL	CENTERLINE	OPNG	OPENING
CP	COMPLETE PENETRATION	OPP	OPPOSITE
CLR	CLEAR	OR	OUTSIDE RADIUS
COL	COLUMN	OWSJ	OPEN WEB STEEL JOIST
CONC	CONCRETE	PCP	PRECAST CONCRETE PANEL
CONN	CONNECTION	PERP	PERPENDICULAR
CONT	CONTINUOUS	PL	PLATE
COORD	COORDINATE	±	PLUS / MINUS
CTR	CENTRE	PRCST	PRECAST
CTRL JT	CONTROL JOINT	PROC	PROCESS
DEMO	DEMOLISH	PSI	POUNDS PER SQUARE INCH
DET	DETAIL	PT CONC	POST-TENSIONED CONCRETE
DIA	DIAMETER	QTY	QUANTITY
DIAG	DIAGONAL	R	RADIUS
DIM	DIMENSION	R/W	REINFORCED WITH
DIST	DISTANCE	REF	REFERENCE
DL	DEAD LOAD	REINF	REINFORCEMENT
-DO-	REPEAT INSTRUCTION	REQD	REQUIRED
DWLG	DRAWING	RWL	RAIN WATER LEADER
DWL	DOWEL	SW	SHORT WAY
EF	EACH FACE	SCD	SCUPPER DRAIN
EJ	EXPANSION JOINT	SCHED	SCHEDULE
ELEC	ELECTRICAL	SECT	SECTION
ELF	ELEVATION	SF	STEP FOOTING
EMB	EMBLEMENT	SHT	SHEET
EQ	EQUAL	SIM	SIMILAR
EQUIP	EQUIPMENT	SOG	SLAB ON GRADE
EQUIV	EQUIVALENT	SPEC	SPECIFICATION
E/S	EACH SIDE	SPMDD	STANDARD PROCTOR
EX	EXISTING		MAXIMUM DRY DENSITY
EXP	EXPANSION	SS	STAINLESS STEEL
EXT	EXTERIOR	STIF	STIFFENER
EW	EACH WAY	STIR	STIRRUP
FD	FLOOR DRAIN	STD	STANDARD
FDN	FOUNDATION	STR	STRINGER
FMC	FULL MOMENT CONNECTION	STAG	STAGGERED
FRMG	FRAMING	STRUCT	STRUCTURAL
FTG	FOOTING	SYMM	SYMMETRICAL
GA	GAUGE	TEMP	TEMPORARY
GALV	GALVANIZED	THK	THICK
GIR	GIRDER	TI	TOP OF
GL	GRID LINE	THRU	THROUGH
GR	GRADE	TJ	TIE JOIST
GR BM	GRADE BEAM	TLL	TOP LOWER LAYER
HC	HOLLOW CORE	TUL	TOP UPPER LAYER
HCMU	HOLLOW CONCRETE MASONRY UNIT	TYP	TYPICAL
HEF	HORIZONTAL EACH FACE	T&B	TOP AND BOTTOM
HIF	HORIZONTAL INSIDE FACE	UNO	UNLESS NOTED OTHERWISE
HOF	HORIZONTAL OUTSIDE FACE	US	UNDERSIDE
H1E	HOOK ONE END	VB	VERTICAL BRACING
H2E	HOOK TWO ENDS	VEF	VERTICAL EACH FACE
HL	HIGH LEVEL	VERT	VERTICAL
HORIZ	HORIZONTAL	VIF	VERTICAL INSIDE FACE
HP	HIGH POINT	VOF	VERTICAL OUTSIDE FACE
HSS	HOLLOW STRUCTURAL STEEL	W/	WITH
ID	INTERNAL DIAMETER	W/O	WITHOUT
IF	INSIDE FACE	WPL	WALL PLATE
IFC	ISSUED FOR CONSTRUCTION	WWM	WELDED WIRE MESH
IFT	ISSUED FOR TENDER	XBR	CROSS BRACE
INT	INTERIOR	XSECT	CROSS SECTION
INV	INVERT		
KO	KNOCK OUT		

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0	12/19/2025	ISSUED FOR TENDER AND PERMIT	S.Y.
No.	Date	Description	By

STAMP:



DESIGNED BY

APPROVED BY

CONSULTANT(S):

ENGINEER:



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PROJECT NAME:

BAYVIEW HEIGHTS PS
ELEVATOR RENOVATION

SHEET TITLE:

GENERAL NOTES

DISCIPLINE:

STRUCTURAL

DRAFTER:	K.L.	SCALE:	AS NOTED
DESIGNER:	B.S.	DATE:	12/19/2025
APPROVER:	S.Y.	CHECKER:	S.Y.
PROJECT No:	A0001195	DRAWING No:	S100
SHEET No:	of		

TITLEBLOCK 24x36 VERT ENG

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1

BLOCK LINTEL SCHEDULE

WALL THK	UP TO 1200 mm		1200 mm TO 2030 mm	
	REINF.	D	REINF.	D
140 mm	1 -10M T&B	390 mm	1 -10M T&B	390 mm
190 mm	1 -10M T&B	390 mm	1 -10M T&B	390 mm
240 mm	1 -10M T&B	390 mm	2 -10M T&B	390 mm

1. MIN BEARING FOR BLOCK LINTEL SHALL BE 200 UNO.

2. FILL VOIDS OR LINTEL BLOCKS W/ 20 MPa CONC. MORTAR IS NOT ACCEPTABLE.

3. WHEN 'T' IS SMALLER THAN 'I', ABOVE LINTEL SCHEDULE DOES NOT APPLY. REFER TO PLAN OR THE STEEL LINTEL SCHEDULE.

40

40

D'

40

40

D

U/S OF FLOOR SLABS, BEAMS & ANY OTHER SOURCE OF LOADING BEARING ON MASONRY WALL.

OPENING OR RECESS IN WALL

MASONRY WALL

I'

T'

BLOCK LINTEL LENGTH [LB]

50

S

C

S

50

TOP BAR

BOT. BAR

200

C/2

200

CLEAR SPAN

2

LINTEL STEEL SCHEDULE

CLEAR SPAN	UP TO 1200 mm	1200 mm TO 1800 mm	1800 mm TO 2100 mm	ARRANGEMENT
90 WALL	L89x89x7.9	L127x89x7.9	L152x89x7.9	
140 WALL	2 - L64x64x7.9	2 - L89x64x7.9	W200x27 + PL 6	
190 WALL	2 - L89x89x7.9	2 - L127x89x7.9	2L - 152x89x7.9	
240 WALL	2 - L102x102x7.9	2 - L152x102x7.9	2L - 152x102x7.9	
290 WALL	2 - L89x89x7.9	3 - L127x89x7.9	3L - 152x89x7.9	
UP TO 3200	W200x27 + PL 6 THK IN CENTER OF WALL			SEE DETAIL

1. PAIR OF LINTEL ANGLES TO BE STITCH WELDED (T&B) @ 600 mm C/C

2. MIN BEARING FOR STEEL ANGLES SHALL BE 150 mm, UNO

3. FOR LINTELS ABUTTING STEEL COLS, CONC WALLS OR OTHER COLS PROVIDE L89x89x9.5 FASTENED TO ABUTMENT.

4. ALL ANGLES SHALL BE LLV, UNO.

5. ALL LOOSE ANGLES SHALL BE HOT DIPPED GALVANIZED, UNO.

CLEAR SPAN	ARRANGEMENT	REMARKS
2100 TO 3600	W200x27 + PL 6 THK. IN CENTER OF WALL	SEE DETAIL

NOTE:
MINIMUM BEARING FOR STEEL ANGLES SHALL BE 300mm, UNO.

WALL VERTICAL REBAR

16Ø x 100 LONG HDG NELSON STUDS WELDED TO BEAM FLANGE @ 600 o/c AND GROUTED TIGHT INTO BLOCK

CUT BLOCK TO SUIT

W200x27 HDG

PROVIDE MASONRY HDG CLIPS ON BEAM WEB @ EVERY SECOND BLOCK

6 THK. CONTINUOUS GALVANIZED PLATE WELDED TO W200. WIDTH TO SUIT BLOCK WALL. BEARING 200 MIN. ON EACH SIDE.

STEEL LINTEL FOR OPENINGS BETWEEN 2100 & 3600

3

BLOCK WALL REBAR LAP

S200

SCALE: 1 : 20

FOR WALL REINF SIZE & SPACING, SEE PLANS & SECTIONS

MASONRY LIFT HEIGHT = 1500

750

LAP SPLICE

MAX REBAR LENGTH PER LIFT

REPEAT THROUGH FULL HEIGHT OF WALL

	LAP SPLICE	MAX REBAR LENGTH
10M	500	2000
15M	700	2200
20M	850	2350
25M	1100	2600

4

TYPICAL BLOCK WALL REINFORCEMENT

S200

SCALE: 1 : 20

NOTE: PREFABRICATED 'T' & 'L' REINF FOR CORNERS MUST BE PROVIDED

FULL HEIGHT VERT BARS, SIZE TO MATCH MAIN VERT REINF

LADDER REINF OR EQUAL @ 400 OC VERT SPACING, GALV

INTERSECTING WALLS

NOTE: BLOCK REINF MUST BE LAPPED ONE COMPLETE SQUARE SECTION OF REINF AS SHOWN

LADDER REINF OR EQUAL @ 400 OC VERT SPACING, GALV

AT OPENINGS AND CONTROL JOINTS

6

SLAB THICKENING UNDER BLOCK WALLS

S200

SCALE: 1 : 20

PROVIDE WALL LATERAL SUPPORT. SEE TYPICAL DETAIL

BOND BEAM REINF. W/ 2-15M CONT.

SEE PLANS FOR REINF.

NON LOAD BEARIG MASONRY PARTITION WALL

10M @ 300 DRILL AND GROUT 75mm C/W HILTI HIT-HY 200 V3

NEW SLAB

EX. SLAB

15M DWLS AT VERT BAR LOCATION

10M @ 300

6 MIL POLYETHYLENE SHEET

50 THK MUD SLAB

150

150

450

150

150

7

DETAIL

S200

SCALE: 1 : 10

T

T/3

T/3

T/3

10 CONT. REBAR TIED TO WATERSTOP AND MAIN REINFORCEMENT @ 500 o/c. TYP.

CONSTRUCTION JOINT WITH PVC WATERSTOP. TYP. SEE PROJECT SPECIFICATION

100

T/6 15 MIN. 50 MAX.

FOR REINFORCEMENT PLACEMENT SEE OTHER SECTIONS

TYPICAL WATERSTOP INSTALLATION DETAILS AT THE INTERSECTION OF SLABS AND WALLS

9

DETAIL

S200

SCALE: 1 : 5

CL

NEW CLIP ANGLE

NEW STEEL BEAM SEE FRAMING PLAN

EXISTING STEEL DECK, SEE PLAN

TOP OF EXISTING STEEL

TOP OF EXISTING STEEL

EXISTING STEEL BEAM SEE FRAMING PLAN

3 SIDES

10

DETAIL

S200

SCALE: 1 : 5

CL

NEW CLIP ANGLE

NEW STEEL BEAM SEE FRAMING PLAN

EXISTING STEEL DECK, SEE PLAN

TOP OF EXISTING STEEL

TOP OF EXISTING STEEL

EXISTING JOIST SHOE TO REMAIN TO SUPPORT EXISTING STEEL DECK

EXISTING STEEL BEAM SEE FRAMING PLAN

3 SIDES

1 BLOCK LINTEL SCHEDULE

2

LINTEL STEEL SCHEDULE

S200

SCALE: 1 : 20

CLEAR SPAN	ARRANGEMENT	REMARKS
2100 TO 3600	W200x27 + PL 6 THK. IN CENTER OF WALL	SEE DETAIL

NOTE:
MINIMUM BEARING FOR STEEL ANGLES SHALL BE 300mm, UNO.

WALL VERTICAL REBAR

16Ø x 100 LONG HDG NELSON STUDS WELDED TO BEAM FLANGE @ 600 o/c AND GROUTED TIGHT INTO BLOCK

CUT BLOCK TO SUIT

W200x27 HDG

PROVIDE MASONRY HDG CLIPS ON BEAM WEB @ EVERY SECOND BLOCK

6 THK. CONTINUOUS GALVANIZED PLATE WELDED TO W200. WIDTH TO SUIT BLOCK WALL. BEARING 200 MIN. ON EACH SIDE.

STEEL LINTEL FOR OPENINGS BETWEEN 2100 & 3600

5

WALL CORNER & INTERSECTION

S200

SCALE: 1 : 20

STANDARD HOOK

CLASS 'B' TENSION LAP SPLICE

STANDARD HOOK

INTERSECTING WALLS

WALL CORNER

8

ELEVATOR HOIST BEAM POCKET

S200

SCALE: 1 : 5

275

25

25

275

275x100x150 WIDE POCKET. GROUT SOLID AFTER BEAM INSTALLATION.

W200x27 BEAM OR AS SPECIFIED ON ELEVATOR SHOP DWGS.

25mm GROUT PAD

ELEVATOR SHAFT WALL

11

BLOCK WALL CONTROL JOINT

S200

SCALE: 1 : 10

SEALANT AND BACKER ROD

COMPRESSIBLE JOINT FILLER

LADDER REINFORCING, DISCONTINUOUS AT CONTROL JOINTS

FULL HEIGHT VERTICAL MAIN BARS TO MATCH SIZES ON PLAN GROUTED SOLID INTO CORES

15M x 300 LONG @ EVERY 400 o/c IN GREASED PVC SLEEVE IN GROUTED CELLS TO PERMIT IN-PLANE HORIZONTAL MOVEMENT. NOTCH BLOCK AS REQUIRED.

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No.	Date	ISSUED FOR TENDER AND PERMIT	S.Y.
0	12/19/2025		
No.	Date	Description	By

STAMPS:

LICENSED PROFESSIONAL ENGINEER
S. L. YOUNG
100212163
12/19/2025
PROVINCE OF ONTARIO

DESIGNED BY

APPROVED BY

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CLIENT:

BAYVIEW HEIGHTS PS
1400 GARVOLIN AVE PICKERING,
ON L1W 1J6

PROJECT NAME:

**BAYVIEW HEIGHTS PS
ELEVATOR RENOVATION**

SHEET TITLE:

TYPICAL DETAILS

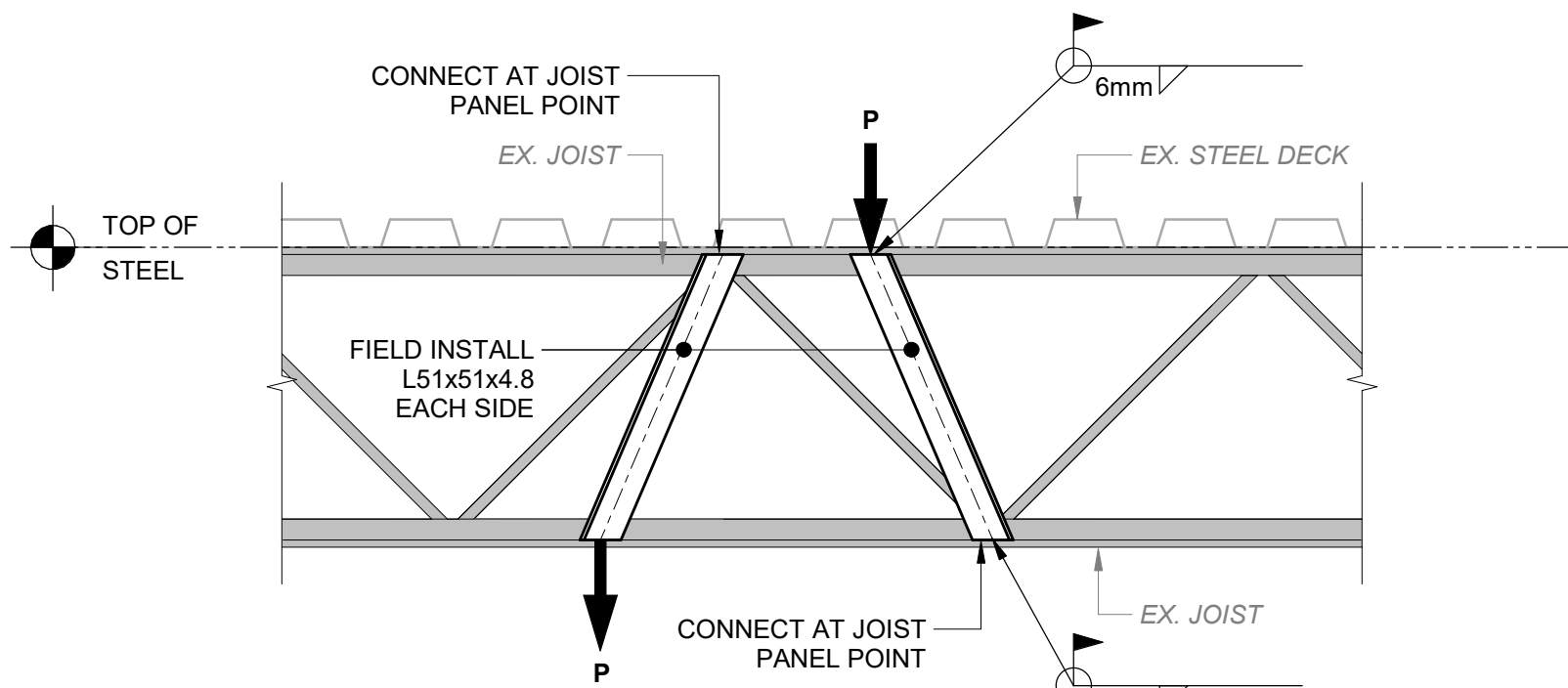
DISCIPLINE:

STRUCTURAL

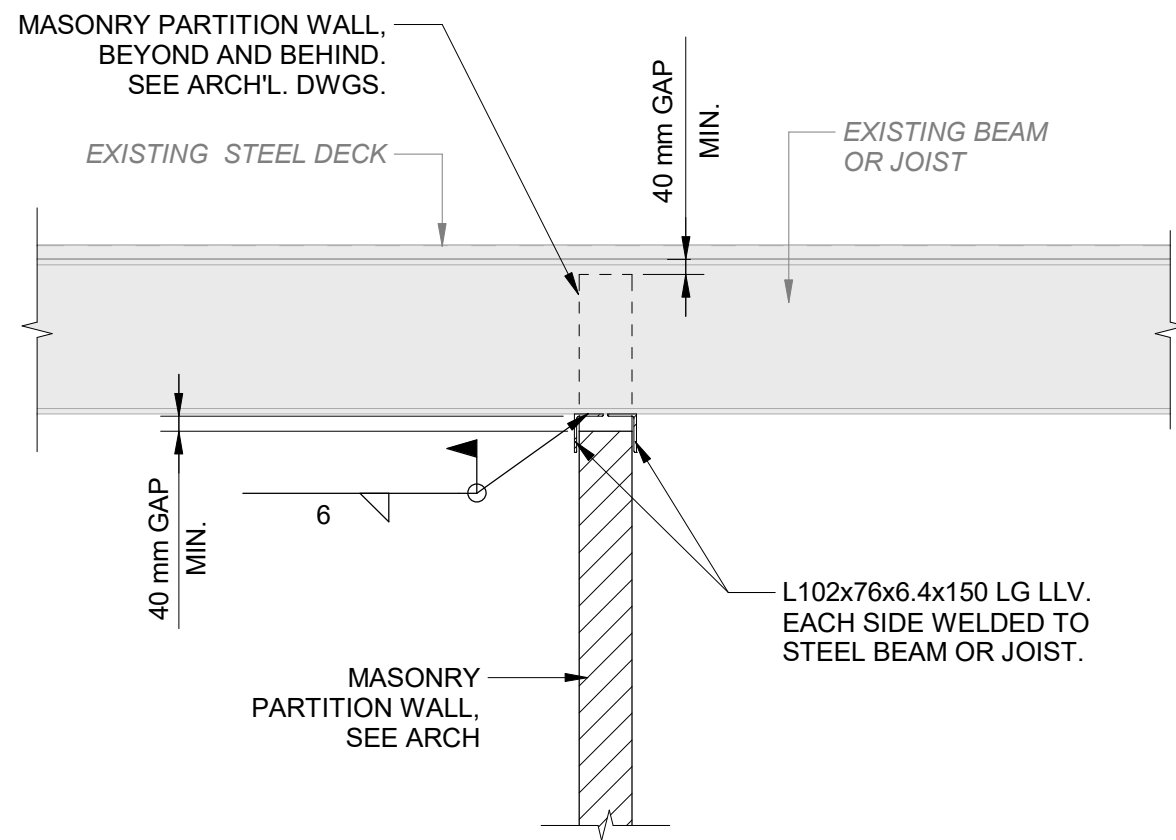
DRAFTER:	K.L.	SCALE:	AS NOTED
DESIGNER:	B.S.	DATE:	12/19/2025
APPROVER:	S.Y.	CHECKER:	S.Y.
PROJECT No:	A0001195	DRAWING No:	S200
SHEET No:	of		

TITLEBLOCK 24x38 VERT ENG

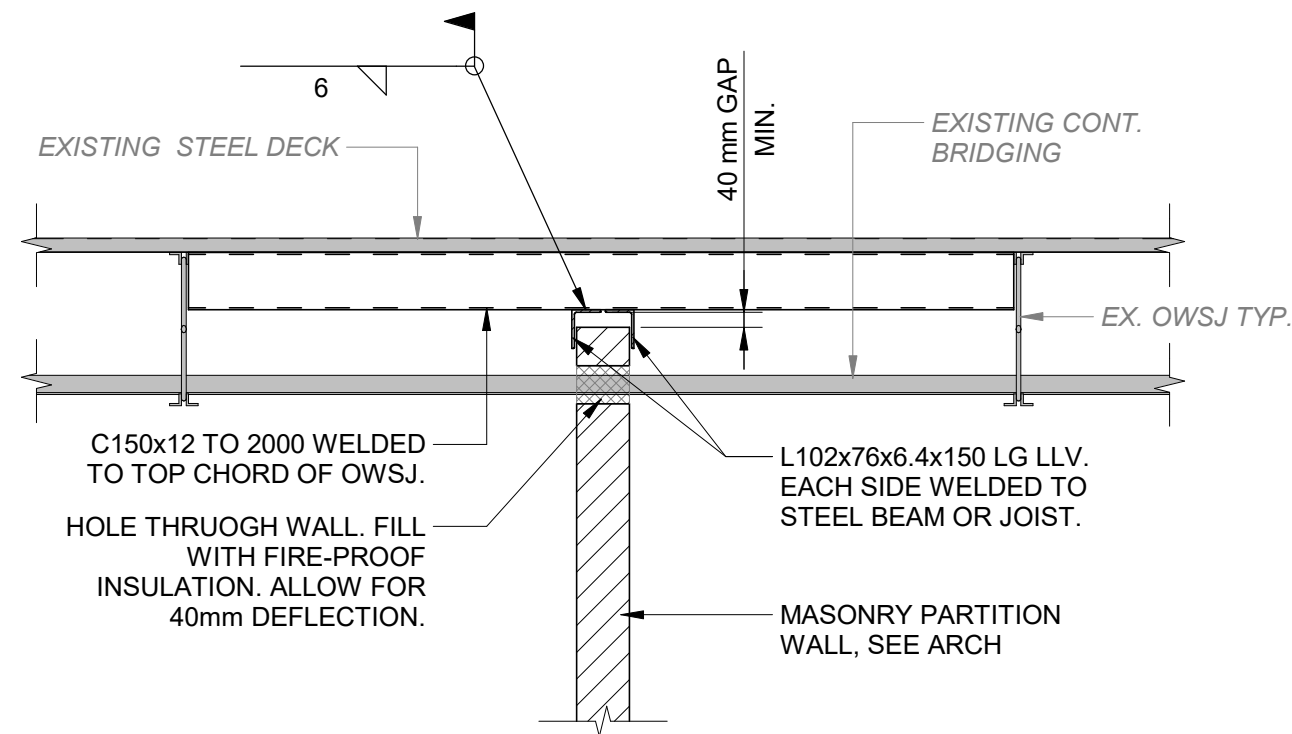
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1 EXISTING JOIST REINFORCING
S201 SCALE: 1 : 10

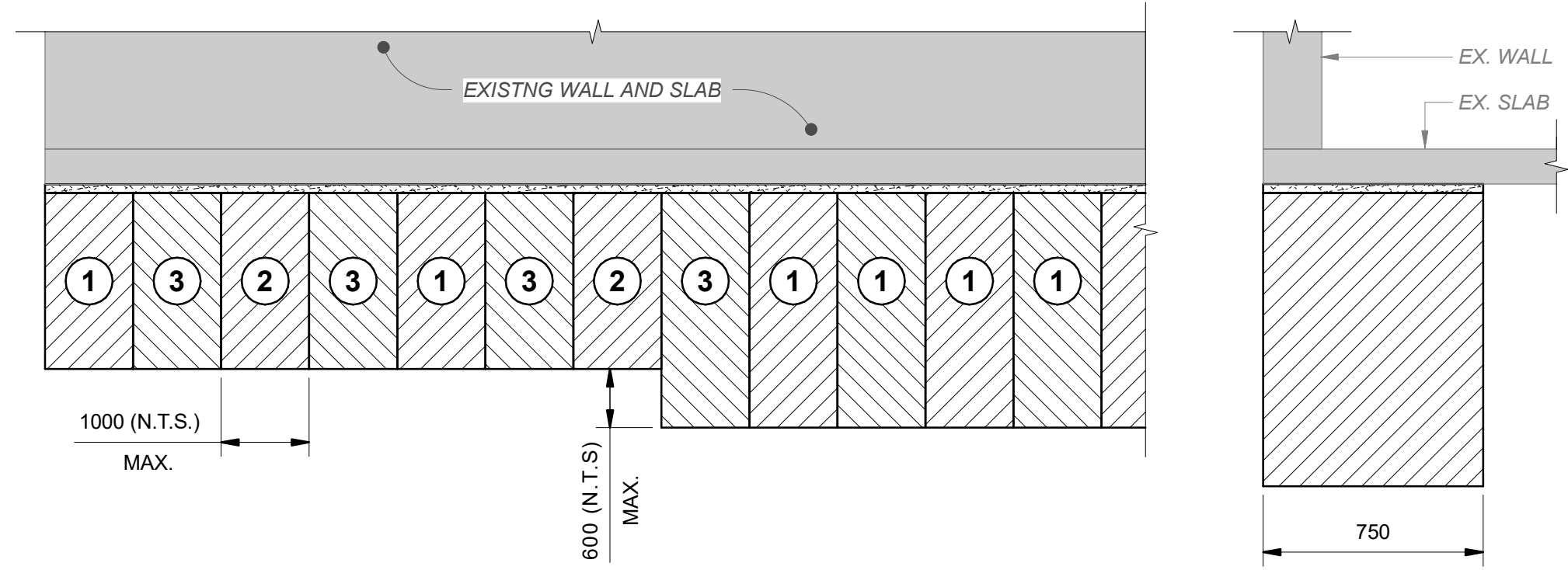


2 LATERAL SUPPORT OF BLOCK WALL
PERPENDICULAR TO BEAMS OR JOISTS
S201 SCALE: 1 : 20



3 LATERAL SUPPORT OF
BLOCK WALL PARALLEL TO JOISTS
S201 SCALE: 1 : 20

UNDERPINNING DETAILS



ELEVATION

ELEVATION

WORK PROCEDURE:

- PROVIDE SAFETY BERM ALONG CONCRETE FOUNDATION WALL.
- EXCAVATE SECTION (1), PROVIDE BREAST BOARDING FOR BACK AND SIDE SUPPORT IF NATURE OF SOIL REQUIRES IT. LAY REINFORCING IF SHOWN ON STRUCTURAL DRAWINGS; PLACE 25 MPa CONCRETE AND ALLOW TO SET FOR AT LEAST 48 HOURS.
- RAM 75mm DRY-PACK EXPANDING GROUT UNDER EXISTING FOUNDATION AND ALLOW TO SET AT LEAST 48 HOURS.
- AFTER ALL SECTION (1) IS COMPLETED, PROCEED WITH SECTION (2) AND UNDERPIN AS DESCRIBED IN PROCEDURE 2. AND 3.
- AFTER ALL SECTION (2) IS COMPLETED, PROCEED WITH SECTION (3) AND UNDERPIN AS DESCRIBED IN PROCEDURE 2. AND 3.
- UNDERPINNING SHALL NOT STEP VERTICALLY GREATER THAN 600mm.
- 7 TO 10 SLOPE FROM ADJACENT EXCAVATION SHALL BE MAINTAINED.
- CONCRETE $f_c = 20$ MPa, TYPE 10 CEMENT
SLUMP 100 MAX.
AGGREGATE 40 MAX.

TENSION LAP SPLICES - CLASS "A" (LTA)

BAR SIZE	TYPE 1 - f_c (MPa)								TYPE 2 - f_c (MPa)							
	20	25	30	35	40	45	50	20	25	30	35	40	45	50	20	25
10M	370	330	300	280	260	250	240	490	440	400	370	350	330	310	490	440
15M	520	470	430	390	370	350	330	690	620	570	520	490	460	440	690	620
20M	630	570	520	480	450	420	400	840	750	690	640	600	560	530	840	750
25M	1020	910	830	770	720	680	650	1360	1210	1110	1030	960	910	860	1360	1210
30M	1210	1080	990	910	860	810	770	1610	1440	1320	1220	1140	1070	1020	1610	1440
35M	1440	1290	1180	1090	1020	960	910	1920	1720	1570	1450	1360	1280	1220	1920	1720
45M	TENSION LAP SPLICES NOT PERMITTED								TENSION LAP SPLICES NOT PERMITTED							
55M	TENSION LAP SPLICES NOT PERMITTED								TENSION LAP SPLICES NOT PERMITTED							

TENSION LAP SPLICES - CLASS "B" (LTB)

BAR SIZE	TYPE 1 - f_c (MPa)								TYPE 2 - f_c (MPa)							
	20	25	30	35	40	45	50	20	25	30	35	40	45	50	20	25
10M	480	430	390	360	340	320	300	640	570	520	480	450	430	400	640	570
15M	670	600	550	510	480	450	430	900	800	730	680	640	600	570	900	800
20M	820	740	670	620	580	550	520	1090	980	890	830	770	730	690	1090	980
25M	1320	1180	1080	1000	940	880	840	1760	1580	1440	1330	1250	1180	1120	1760	1580
30M	1570	1400	1280	1190	1110	1050	990	2090	1870	1710	1580	1480	1400	1320	2090	1870
35M	1870	1680	1530	1420	1330	1250	1190	2500	2230	2040	1890	1770	1670	1580	2500	2230
45M	TENSION LAP SPLICES NOT PERMITTED								TENSION LAP SPLICES NOT PERMITTED							
55M	TENSION LAP SPLICES NOT PERMITTED								TENSION LAP SPLICES NOT PERMITTED							

TENSION LAP SPlice NOTES

- REFERENCE CODE CAN/CSA A23.3-19.
- TABLES ARE IN MILLIMETERS, ϕ_b = BAR DIAMETER.
- TENSION LAP SPLICES:
 - CLASS "A": APPLICABLE WHEN REINFORCING PROVIDED IS AT LEAST TWICE THAT REQUIRED BY ANALYSIS AND LESS THAN ONE-HALF OF THE TOTAL REINF. IS SPLICED WITHIN THE REQUIRED LAP LENGTH.
 - CLASS "B": APPLICABLE EVERYWHERE EXCEPT WHERE CLASS "A" CAN BE USED.
 - TENSION LAP SPlice LENGTH SHALL NOT BE LESS THAN 300mm.
 - TENSION LAP SPLICES FOR BARS LARGER THAN 35M ARE NOT PERMITTED.
 - TYPES 1 AND 2 ARE THE SAME AS TENSION DEVELOPMENT TYPES - SEE NOTES FOR DEVELOPMENT OF REINFORCING BARS.
 - TENSION LAP SPLICES LISTED IN TENSION LAP SPlice TABLES SHALL BE MULTIPLIED WITH THE FOLLOWING FACTORS FOR:

MODIFICATION FACTOR

- | | |
|---|-----|
| k1 - TOP BARS HAVING MORE THAN 300mm OF FRESH CONCRETE CAST BELOW THE DEVELOPMENT LENGTH. | 1.3 |
| k2 - FOR EPOXY-COATED REINFORCEMENT WITH CLEAR COVER LESS THAN 3 ϕ_b OR WITH CLEAR SPACING BETWEEN BARS BEING DEVELOPED LESS THAN 6 ϕ_b . | 1.5 |
| - FOR ALL OTHER EPOXY-COATED REINFORCEMENT | 1.2 |
| k3 - FOR STRUCTURAL LOW-DENSITY CONCRETE | 1.3 |
| - FOR STRUCTURAL SEMI-LOW-DENSITY CONCRETE | 1.2 |

THE PRODUCT OF k1k2 NEED TO BE TAKEN GREATER THAN 1.7.

- BARS SPLICED BY NON-CONTACT LAP SPLICES SHALL NOT HAVE A TRANSVERSE SPACING NOT EXCEEDING THE LESSER OF ONE-FIFTH OF THE REQUIRED LAP SPlice LENGTH OR 150mm.
- LAP SPLICES OF BARS IN BUNDLE SHALL BE BASED ON THE LAP SPlice LENGTH REQUIRED FOR INDIVIDUAL BARS WITHIN THE BUNDLE. INCREASED 10% FOR A 2 BAR BUNDLE INCREASED 20% FOR 3 BAR BUNDLES AND 33% FOR A 4 BAR BUNDLE. ENTIRE BUNDLE SHALL NOT BE LAP SPLICED WITHIN ONE LAP LENGTH.

COMPRESSION LAP SPLICES (Lc)

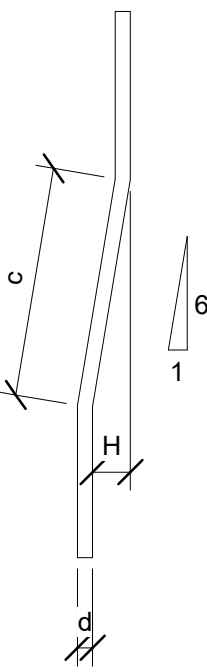
BAR SIZE	fc < 20 MPa			fc > 20 MPa		
	OPEN	ENCLOSED		OPEN	ENCLOSED	
		TIES	SPIRAL		TIES	SPIRAL
10M	440	370	330	330	300	300
15M	630	520	470	470	390	360
20M	760	630	570	570	480	430
25M	980	820	740	740	620	560
30M	1170	970	880	880	730	660
35M	1390	1160	1040	1040	870	790
45M	NOTE 4					
55M	NOTE 4					

NOTES FOR COMPRESSION LAP SPLICES OF REINFORCING BARS

- REF CODE CAN/CSA A23.3-19.
- TABLES ARE IN MILLIMETERS.
- COMPRESSION LAP SPLICES:
 - SHALL NOT BE LESS THAN 300mm
 - WHEN BARS OF DIFFERENT SIZE ARE LAP SPLICED IN COMPRESSION, SPlice LENGTH SHALL BE THE LARGER OF EITHER DEVELOPMENT LENGTH OF THE LARGER BAR, OR SPlice LENGTH OF THE SMALLER BAR. LAP SPLICES OF 35M BARS AND SMALLER BARS ARE PERMITTED TO 45M AND 55M BARS.
 - TIED REINFORCED MEMBERS ARE DEFINED WHERE TIES THROUGHOUT THE LAP SPlice LENGTH HAVE AN EFFECTIVE AREA NOT LESS THAN DEFINED IN CAN/CSA A23.3-19, CLAUSE 12.17.3.4.
- LAP SPLICES NOT PERMITTED, USE WELDED SPLICES OR MECHANICAL CONNECTIONS.

MAXIMUM OFFSET			
BAR SIZE	BAR ϕ (mm)	H (mm)	C (mm)
20M	19.5	60	240
25M	25.2	70	270
30M	29.9	80	300
35M	35.7	90	330
45M	43.7	LAP SPLICES NOT PERMITTED	
55M	56.4		

MAXIMUM OFFSET		
BAR SIZE	BAR ϕ (mm)	H (mm)
ALL BARS	-	75mm PLUS ϕ OF BAR AND BAR ABOVE



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No.	Date	Description	By
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STAMPS:



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APPROVED BY

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BAYVIEW HEIGHTS PS
1400 GARVOLIN AVE PICKERING,
ON L1W 1J6

PROJECT NAME:

BAYVIEW HEIGHTS PS
ELEVATOR RENOVATION

SHEET TITLE:

TYPICAL DETAILS

DISCIPLINE:

STRUCTURAL

DRAFTER:

K.L.

SCALE:

AS NOTED

DESIGNER:

B.S.

DATE:

12/19/2025

APPROVER:

S.Y.

CHECKER:

S.Y.

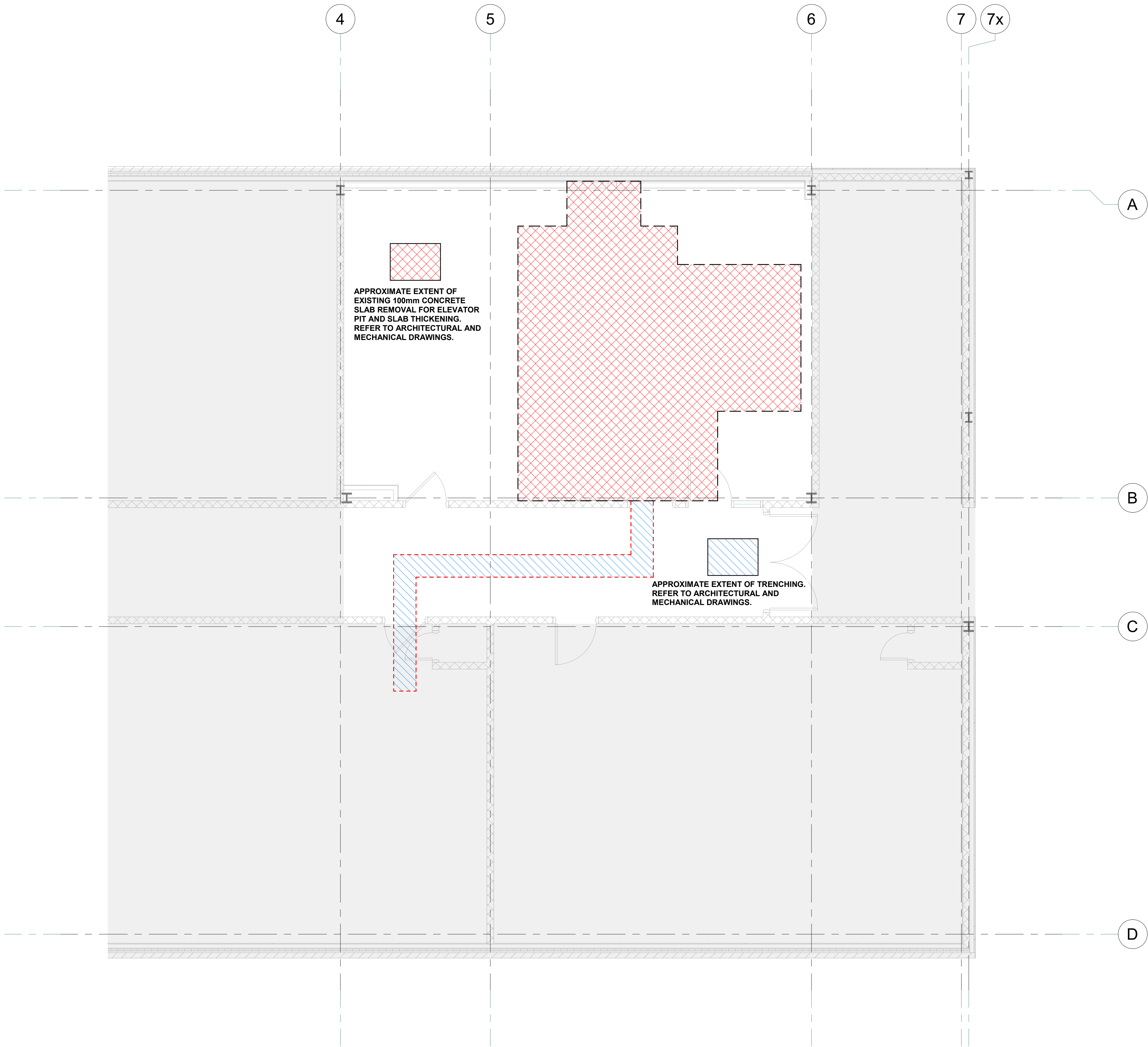
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A0001195

DRAWING No:

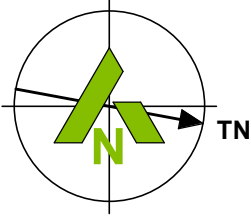
S201

SHEET No:

of



1 SLAB REMOVAL / DEMO PLAN
S300 SCALE: 1 : 50



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1400 GARVOLIN AVE PICKERING,
ON L1W 1J6

PROJECT NAME:

BAYVIEW HEIGHTS PS
ELEVATOR RENOVATION

SHEET TITLE:

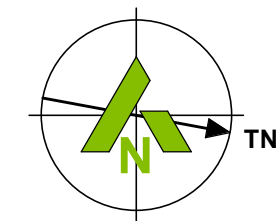
DEOMOLITION INFO - SLAB

DISCIPLINE: STRUCTURAL			
DRAFTER:	K.L.	SCALE:	AS NOTED
DESIGNER:	B.S.	DATE:	12/19/2025
APPROVER:	S.Y.	CHECKER:	S.Y.
PROJECT No:	A0001195	DRAWING No:	S300
SHEET No:	of		



NOTES:

- 1. SHORE EXISTING SECOND FLOOR AND ROOF PRIOR TO JOIST AND DECK REMOVALS.**
- 2. SHORING TO REMAIN IN PLACE UNTIL NEW STEEL FRAMING HAS BEEN INSTALLED.**

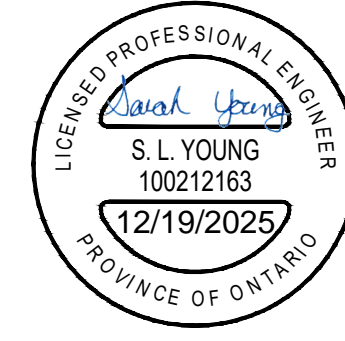


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No.	Date	Description	Bv

STAMP2



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CONSULTANT(S)

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BAYVIEW HEIGHTS PS
1400 GARVOLIN AVE PICKERING,
ON L1W 1J6

PROJECT NAME:

BAYVIEW HEIGHTS PS
ELEVATOR RENOVATION

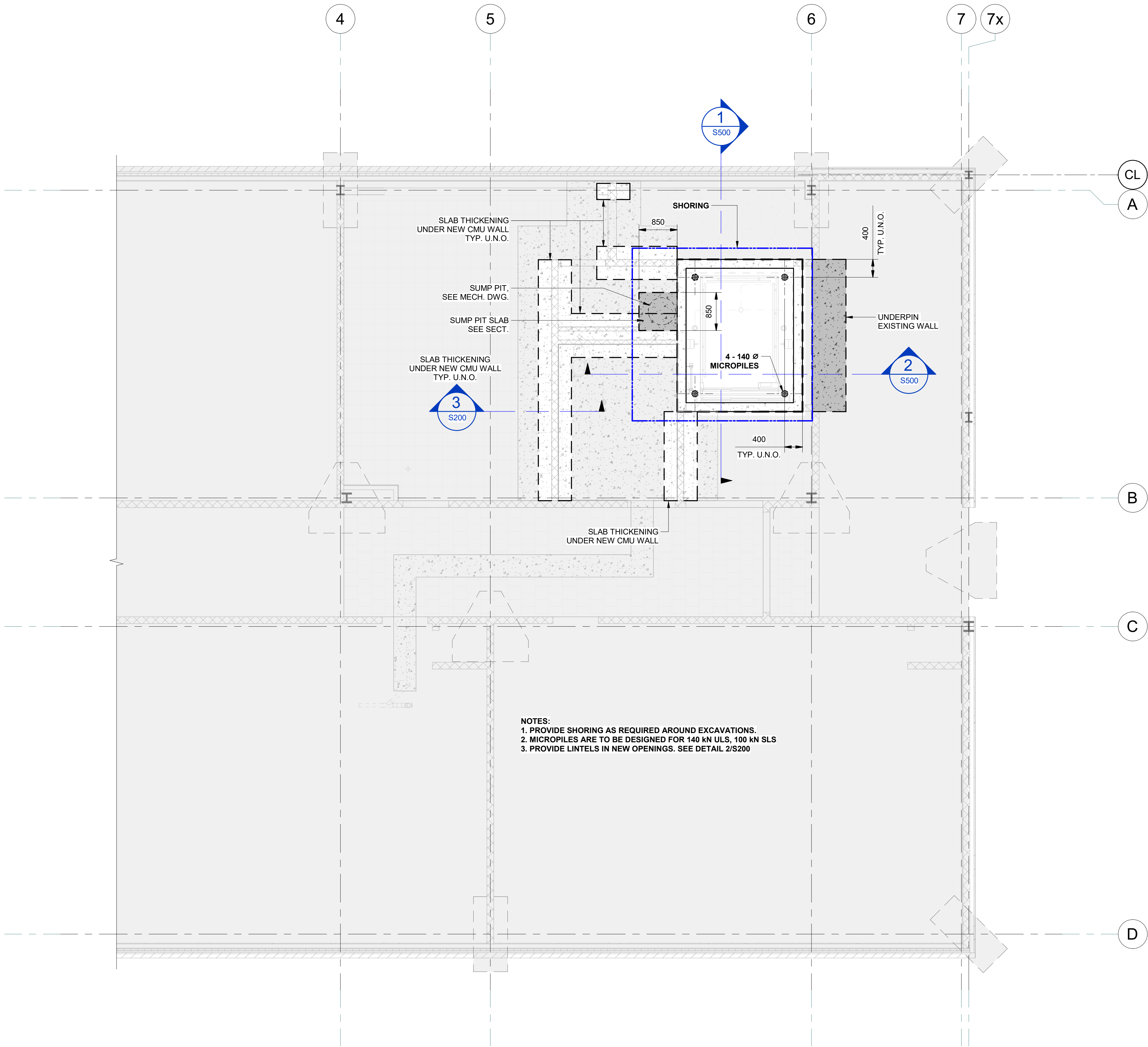
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DEOMOLITION INFO

DISCIPLINE	
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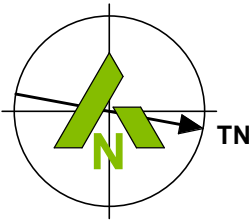
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DRAFTER:	K.L.	SCALE:	AS NOTED
DESIGNER:	B.S.	DATE:	12/19/2025
APPROVER:	S.Y.	CHECKER:	S.Y.
PROJECT No:	A0001195	DRAWING No:	S301
SHEET No:	of		



NOTES:
1. PROVIDE SHORING AS REQUIRED AROUND EXCAVATIONS.
2. MICROPILES ARE TO BE DESIGNED FOR 140 kN ULS, 100 kN SLS
3. PROVIDE LINTELS IN NEW OPENINGS. SEE DETAIL 2/S200

1 PARTIAL EXISTING FOUNDATION PLAN
S400 SCALE: 1 : 50



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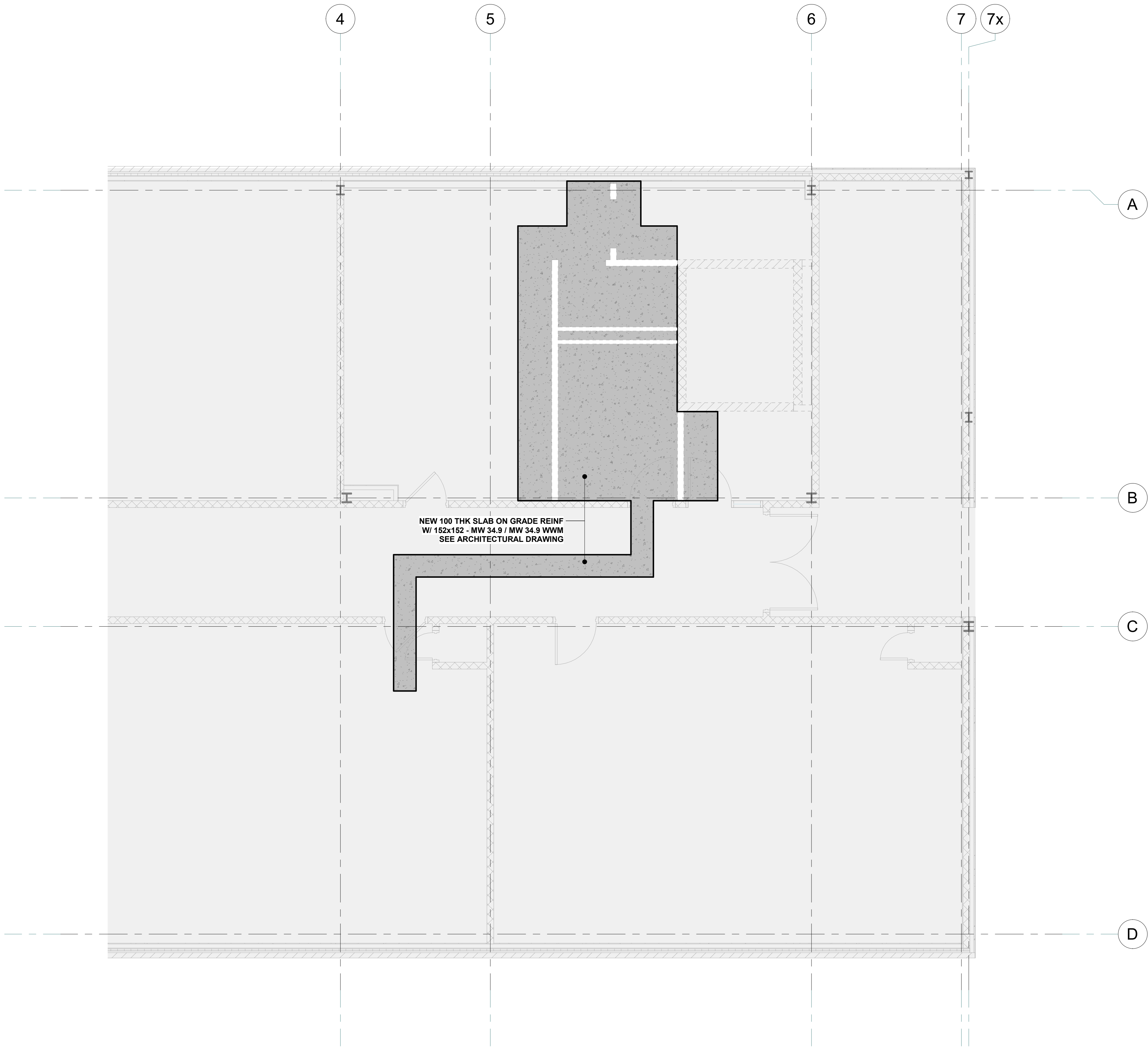
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CLIENT:
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ON L1W 1J6

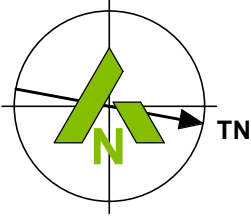
PROJECT NAME:
**BAYVIEW HEIGHTS PS
ELEVATOR RENOVATION**

SHEET TITLE:
**PARTIAL EXISTING FOUNDATION
PLAN**

DISCIPLINE:		STRUCTURAL	
DRAFTER:	K.L.	SCALE:	AS NOTED
DESIGNER:	B.S.	DATE:	12/19/2025
APPROVER:	S.Y.	CHECKER:	S.Y.
PROJECT No:	A0001195	DRAWING No:	S400
SHEET No:	of		

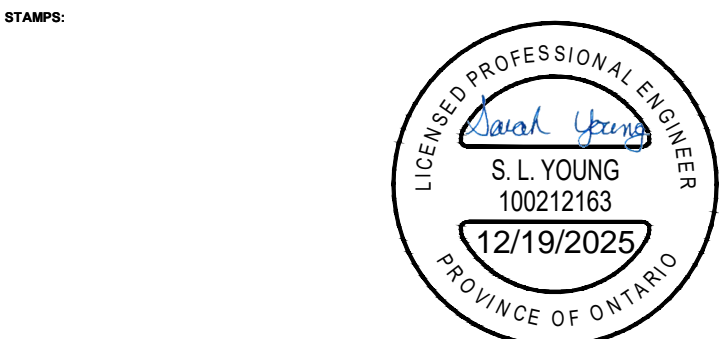


1 PARTIAL NEW SLAB PLAN
S401 SCALE: 1 : 50



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No.	Date	Description	By



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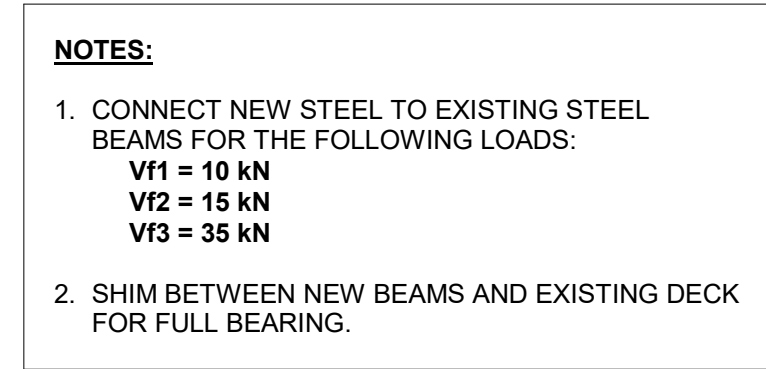
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CLIENT:
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1400 GARVOLIN AVE PICKERING,
ON L1W 1J6

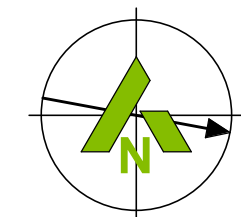
PROJECT NAME:
**BAYVIEW HEIGHTS PS
ELEVATOR RENOVATION**

SHEET TITLE:
NEW PARTIAL SLAB PLAN

DISCIPLINE: STRUCTURAL	
DRAFTER: K.L.	SCALE: AS NOTED
DESIGNER: B.S.	DATE: 12/19/2025
APPROVER: S.Y.	CHECKER: S.Y.
PROJECT No: A0001195	DRAWING No: S401
SHEET No: of	



<u>DESIGN LOADS (UNFACTORED):</u>	
<u>DEAD:</u>	
TILE:	0.07 kPa
2.5" CONCRETE SLAB ON METAL PAN:	1.53 kPa
SUSPENDED CEILING:	0.20 kPa
MECH & ELEC ALLOWANCE:	0.25 kPa
<u>PARTITION ALLOWANCE:</u>	<u>1.00 kPa</u>
TOTAL:	3.05 kPa
<u>PLUS SELF WEIGHT OF STRUCTURAL STEEL</u>	
LIVE:	3.60 kPa

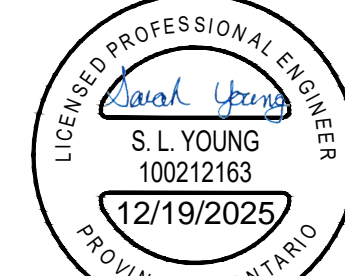


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STAMPS



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APPROVED BY _____

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BAYVIEW HEIGHTS PS
1400 GARVOLIN AVE PICKERING,
ON L1W 1J6

PROJECT NAME:

BAYVIEW HEIGHTS PS ELEVATOR RENOVATION

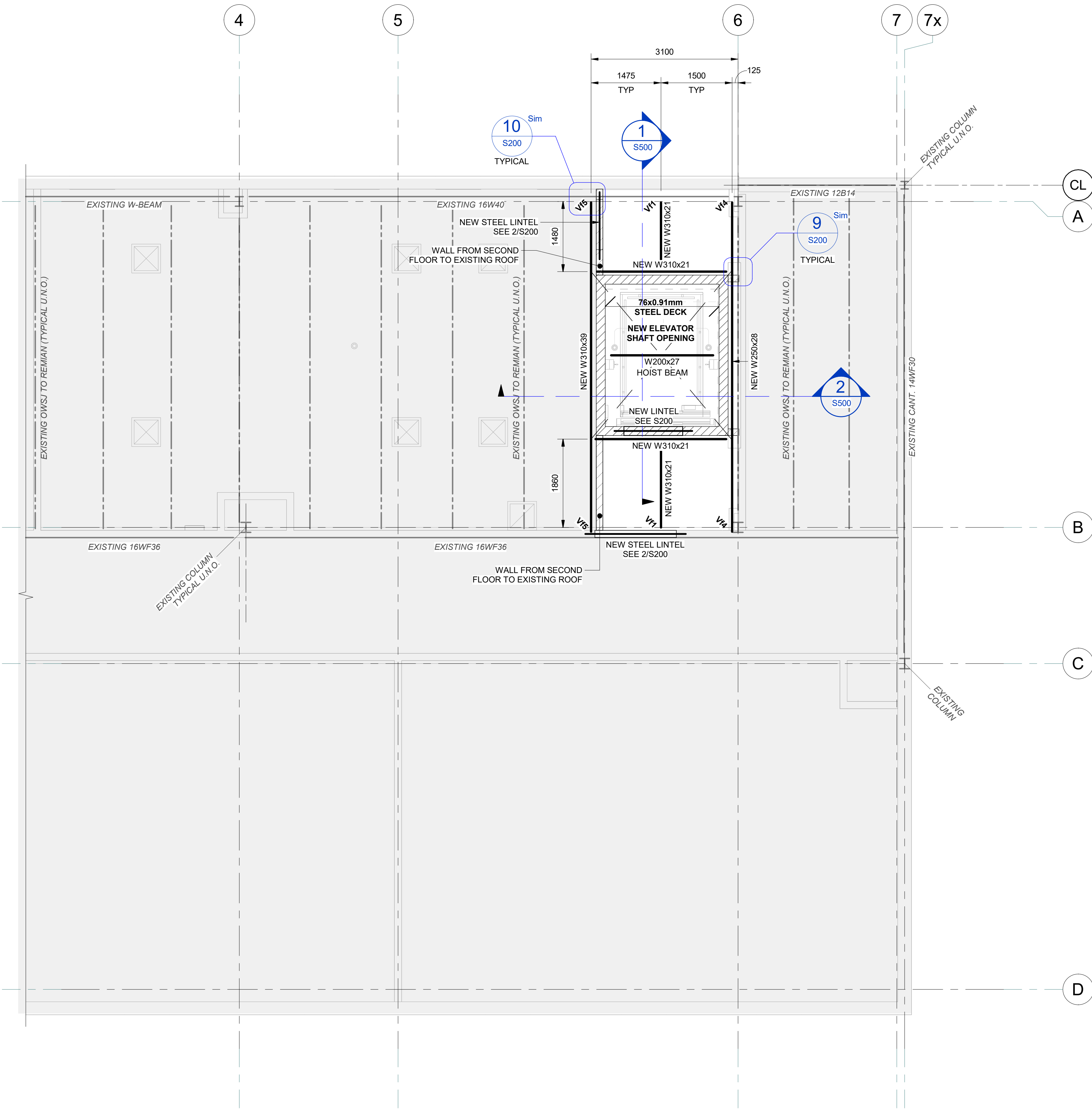
SHEET TITLE:

PARTIAL EXISTING SECOND
FLOOR FRAMING PLAN

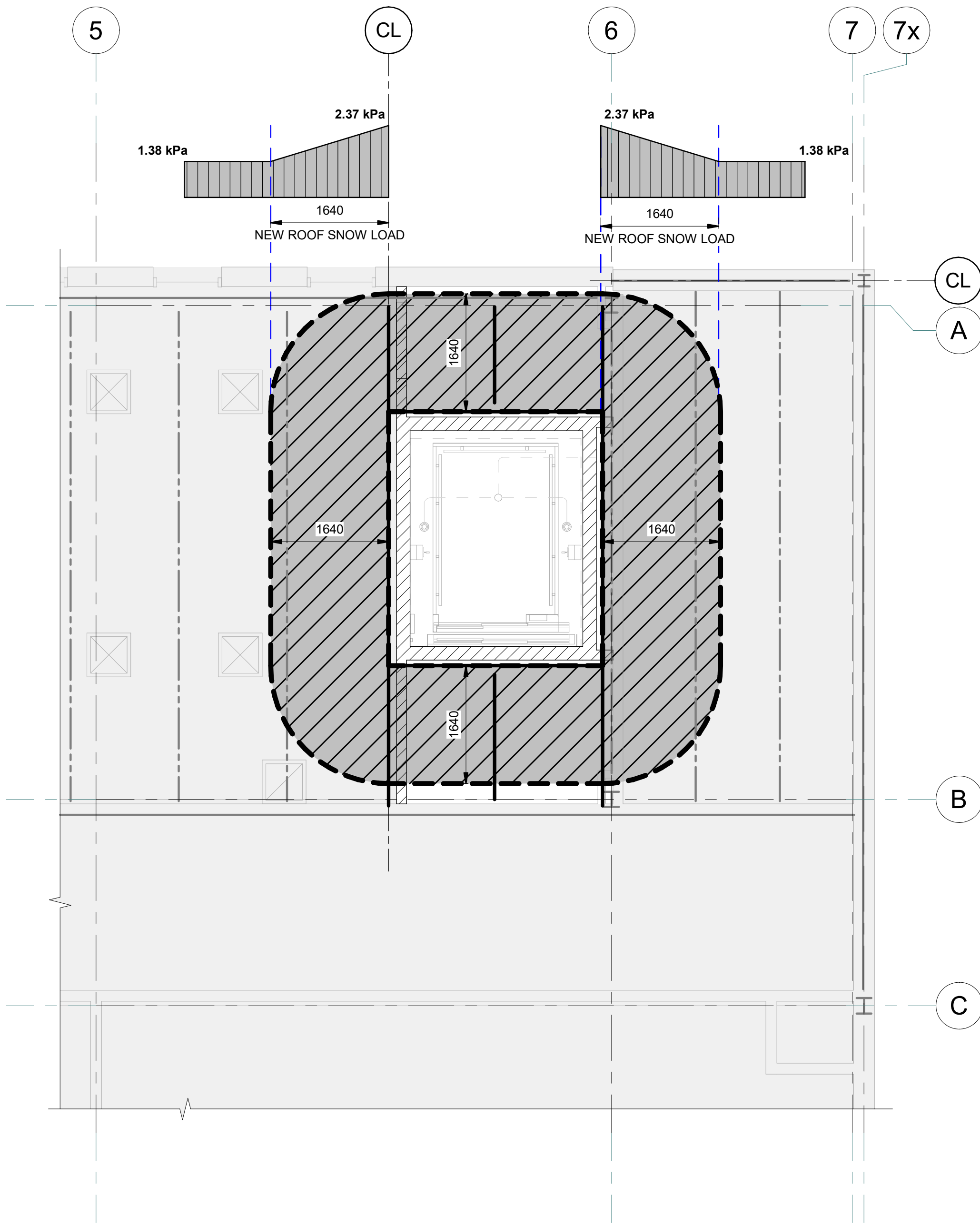
DISCIPLINE	
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STRUCTURAL

DRAFTER:	K.L.	SCALE:	AS NOTED
DESIGNER:		DATE:	12/19/2025
APPROVER:	S.Y.	CHECKER:	S.Y.
PROJECT No:	A0001195	DRAWING No:	S402
SHEET No:	of		



1 PARTIAL EXISTING ROOF FRAMING PLAN
S403 SCALE: 1 : 50

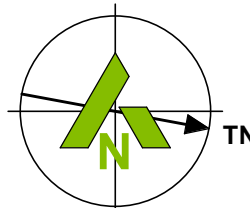


2 SNOW PILE UP DIAGRAM / LOADING PLAN
S403 SCALE: 1 : 50

- NOTES:**
- CONNECT NEW STEEL TO EXISTING STEEL BEAMS FOR THE FOLLOWING LOADS:
V11 = 10 kN
V14 = 20 kN
V15 = 30 kN
 - SHIM BETWEEN NEW BEAMS AND EXISTING DECK FOR FULL BEARING.

EXISTING ROOF DESIGN LOADS (UNFACTORED):	
DEAD:	
ROOFING:	0.32 kPa
METAL DECK:	0.10 kPa
SUSPENDED CEILING:	0.20 kPa
MECH & ELEC ALLOWANCE:	0.25 kPa
TOTAL:	0.87 kPa
PLUS SELF WEIGHT OF STRUCTURAL STEEL	
SNOW:	1.38 kPa
PLUS ACCUMULATION AS SHOWN ON PLAN	

NEW ROOF DESIGN LOADS (UNFACTORED):	
DEAD:	
ROOFING:	0.36 kPa
METAL DECK:	0.15 kPa
MECH & ELEC ALLOWANCE:	0.25 kPa
TOTAL:	0.76 kPa
PLUS SELF WEIGHT OF STRUCTURAL STEEL	
SNOW:	1.38 kPa
PLUS ACCUMULATION AS SHOWN ON PLAN	



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No.	Date	Description	S.Y.
0	12/19/2025	ISSUED FOR TENDER AND PERMIT	S.Y.

STAMPS:



DESIGNED BY: APPROVED BY:

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CLIENT:
BAYVIEW HEIGHTS PS
1400 GARVOLIN AVE PICKERING,
ON L1W 1J6

PROJECT NAME:
**BAYVIEW HEIGHTS PS
ELEVATOR RENOVATION**

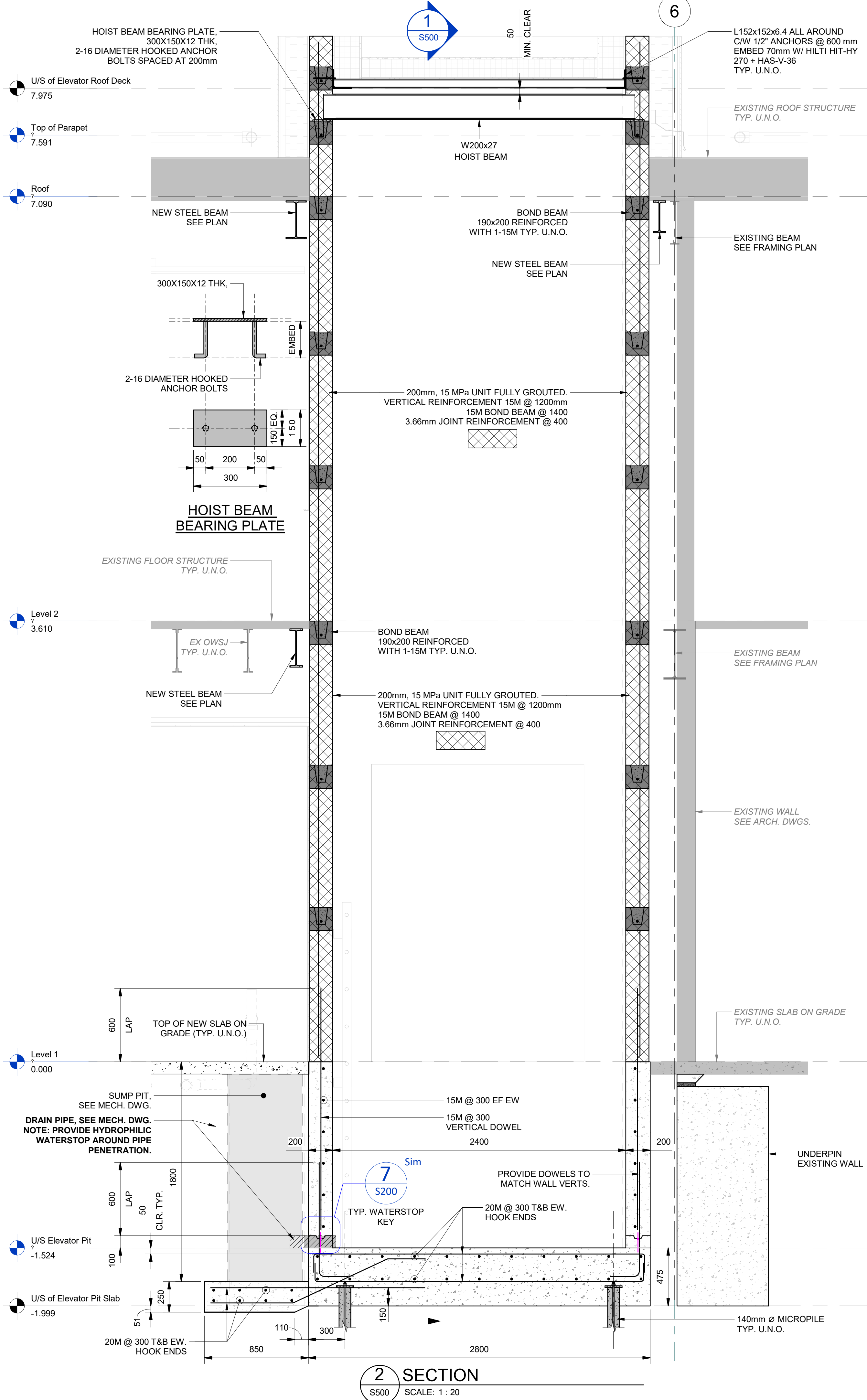
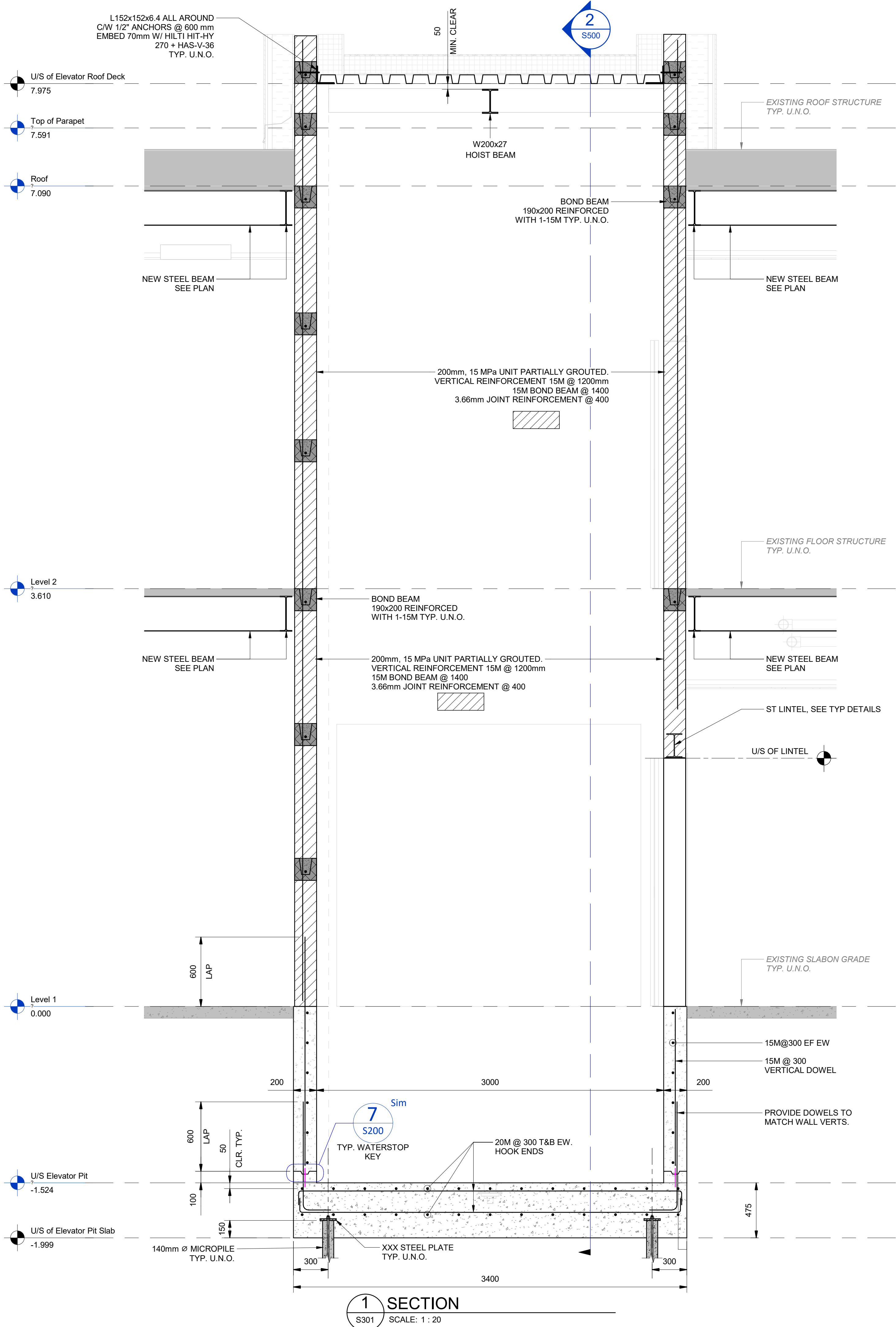
SHEET TITLE:
**PARTIAL EXISTING ROOF FRAMING
PLAN**

DISCIPLINE:
STRUCTURAL

DRAFTER:	K.L.	SCALE:	AS NOTED
DESIGNER:	B.S.	DATE:	12/19/2025
APPROVER:	S.Y.	CHECKER:	S.Y.
PROJECT No:	A0001195	DRAWING No:	S403
SHEET No:	of		

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STAMPS:



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ON L1W 1J6

PROJECT NAME:

BAYVIEW HEIGHTS PS
ELEVATOR RENOVATION

SHEET TITLE:

DETAILS AND SECTIONS

DISCIPLINE:

STRUCTURAL

DRAFTER:

K.L.

SCALE:

AS NOTED

DESIGNER:

B.S.

DATE:

12/19/2025

APPROVER:

S.Y.

CHECKER:

S.Y.

PROJECT No:

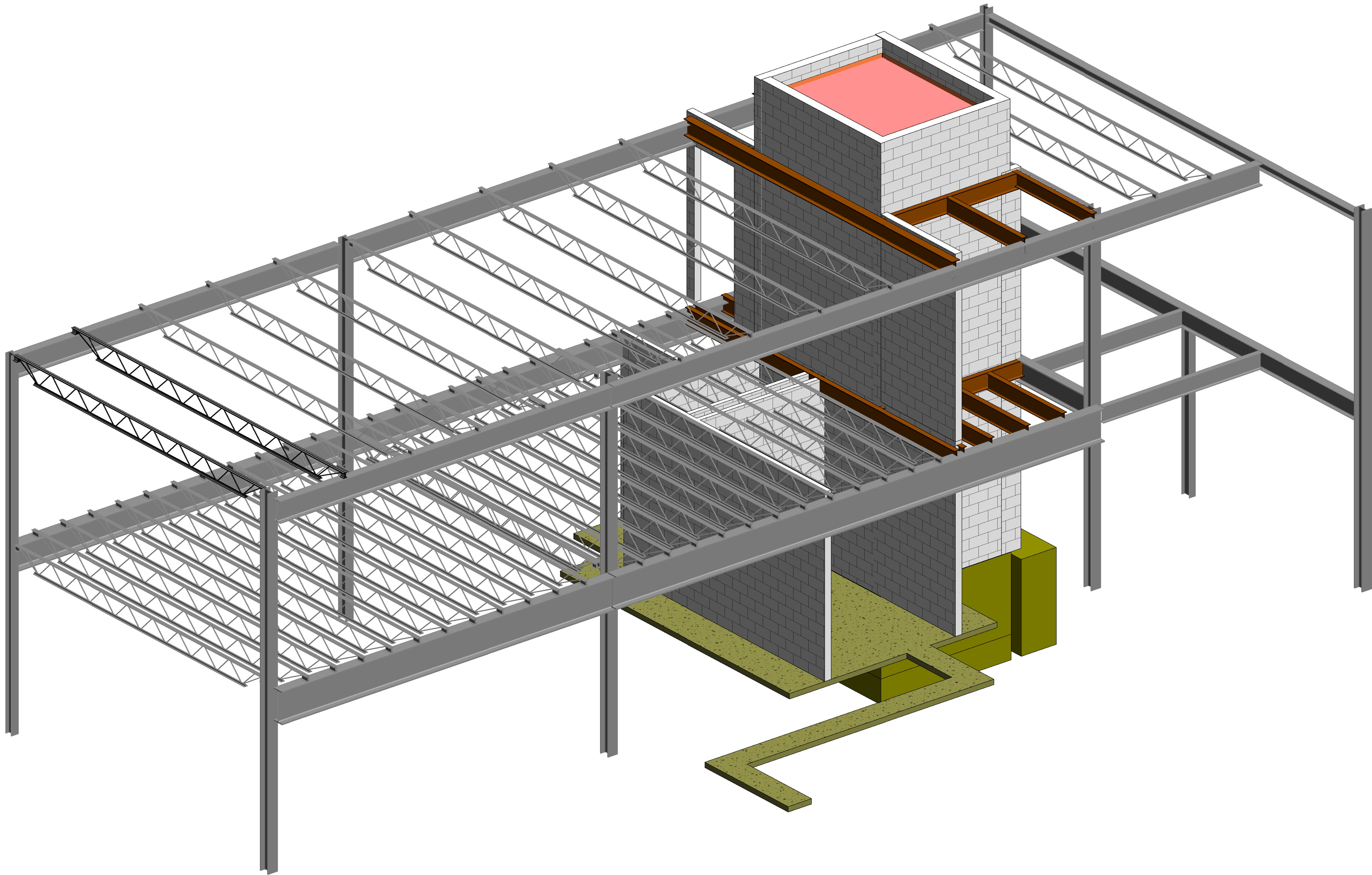
A0001195

DRAWING No:

S500

SHEET No:

of



1 ISOMETRIC 3D VIEW

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No.	Date	Description	By

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PROJECT NAME:
**BAYVIEW HEIGHTS PS
ELEVATOR RENOVATION**

SHEET TITLE:
ISOMETRIC VIEW 3D

DISCIPLINE:		STRUCTURAL	
DRAFTER:	K.L.	SCALE:	AS NOTED
DESIGNER:	B.S.	DATE:	12/19/2025
APPROVER:	S.Y.	CHECKER:	S.Y.
PROJECT No:	A0001195	DRAWING No:	S601
SHEET No:	of		