#### **GENERAL**

- 1. ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE REQUIREMENTS OF THE ONTARIO BUILDING CODE, LATEST EDITION AND THE OCCUPATIONAL HEALTH AND SAFFTY ACT/REGULATIONS FOR CONSTRUCTION PROJECTS
- 2. CONFORM TO OWNER'S GENERAL SPECIFICATIONS INCLUDING ALL SAFETY REQUIREMENTS.
- 3. SITE VERIFY ALL DIMENSIONS AND LEVELS.
- 4. KEEP THE SITE THROUGHOUT THE WORK AREA IN A CLEAN AND ORDERLY CONDITION AT ALL TIMES TO THE SATISFACTION OF THE OWNER.
- 5. THE LATEST EDITION OF ALL CODES AND STANDARDS SHALL BE USED.
- 6. ALL STRUCTURAL DRAWINGS ARE TO BE READ IN CONJUNCTION WITH OTHER CONSULTANTS DRAWINGS.

#### EXCAVATION, BACKFILLING AND GRADING

#### CODES, REGULATIONS

- 1. CONFORM TO THE REQUIREMENTS OF THE ONTARIO BUILDING CODE, THE ONTARIO PROVINCIAL STANDARD SPECIFICATIONS (OPSS) AND THE WORKPLACE HEALTH AND
- 2. PROVIDE MINIMUM 8" TOPSOIL BELOW ANY DISTURBED GREEN SCAPE AREAS TO BE RESTORED PRIOR TO SODDING.

#### **EXAMINATION**

1. BEFORE TENDERING THE WORK, THE CONTRACTOR SHALL EXAMINE THE SITE AND ASCERTAIN THE EXTENT AND NATURE OF THE MATERIAL IT MAY BE NECESSARY TO REMOVE TO PROVIDE FOR THE DEPTHS. LEVELS. AND GRADES REQUIRED. TO ASSURE HIMSELF THAT HIS DETERMINATIONS ARE MADE IN CONFORMITY WITH THE DRAWINGS AND SPECIFICATIONS.

#### SETTING OUT WORK

- 1. THE DRAWINGS INDICATE THE SITE COMPONENTS LOCATION, AND PROPOSED AND FINAL GRADES. BE RESPONSIBLE TO CONSTRUCT THE WORK ACCORDING TO LEVELS AND LOCATIONS SHOWN ON THE DRAWINGS, REPORT ANY FRRORS OR DISCREPANCIES TO THE CONSULTANT BEFORE COMMENCING WITH THE WORK. COMMENCEMENT OF ANY PART OF THE WORK SHALL CONSTITUTE ACCEPTANCE OF DRAWINGS AS BEING CORRECT.
- 2. EMPLOY A COMPETENT INSTRUMENT MAN AND PROVIDE ALL LINES AND LEVELS, LIMIT LINES AND BOUNDARY STAKES FOR THE EXECUTION OF THE WORK AS REQUIRED. ALL BENCH MARKS SHALL BE CAREFULLY PROTECTED
- 3. PROVIDE ALL SUBCONTRACTORS WITH, AND BE RESPONSIBLE FOR, ALL LINES, LEVELS AND DIMENSIONS WHICH SUCH TRADES REQUIRE TO RELATE THEIR WORK TO THE WORK OF THE CONTRACTOR OR OTHER TRADES. ALL TRADES SHALL BE NOTIFIED THAT ALL SUCH LEVELS AND DIMENSIONS MUST BE OBTAINED FROM THE CONTRACTOR.

#### <u>PROTECTION</u>

- A. PROTECTION OF EXISTING SERVICES
- 1. BEFORE STARTING THE WORK, VERIFY THE LOCATION OF ALL KNOWN UNDERGROUND SERVICES AND UTILITIES OCCURRING IN THE WORK SITE AREA.
- 2. NOTIFY THE OWNER, PUBLIC UTILITY OR MUNICIPAL AUTHORITIES IN ADVANCE OF PLANNED EXCAVATIONS ADJACENT TO THEIR SERVICES.
- 3. TAKE CARE NOT TO DAMAGE OR DISPLACE ENCOUNTERED KNOWN AND UNKNOWN
- 4. WHEN SUCH SERVICES ARE ENCOUNTERED DURING THE EXECUTION OF WORK. IMMEDIATELY NOTIFY THE CONSULTANT AND PROTECT, BRACE AND SUPPORT ACTIVE SERVICES. WHERE REPAIRS TO THESE SERVICES BECOME NECESSARY USE THE
  - a. KNOWN SERVICES, REPAIR AT NO EXPENSE TO THE OWNER. b. UNKNOWN SERVICES, FORWARD TO THE CONSULTANT A COMPLETE BREAKDOWN OF THE ESTIMATED COST OF SUCH WORK. PROCEED ONLY UPON WRITTEN AUTHORIZATION.
- 5. IN THE CASE OF DAMAGE TO, OR CUTTING OFF OF AN ESSENTIAL SERVICE, NOTIFY CONSULTANT, THE OWNER, AND PUBLIC UTILITY OR MUNICIPAL AUTHORITIES IMMEDIATELY AND REPAIR THE SERVICE UNDER THE CONSULTANT'S DIRECTION.

ASSUME REUSE EXISTING COMPACT SOIL AND INFILL BALANCE OF GRANULAR MATERIAL/TOPSOIL AS REQUIRED TO ACCOMPLISH WORK ON PROJECT. ALL FILL SHALL BE STORED ON SITE IN A LOCATION APPROVED BY THE OWNER/CONSULTANT AND SHALL BE PROTECTED AS REQUIRED FOR RE-USE.

#### EXCAVATION WORK

- 1. REMOVE ALL CONCRETE, MASONRY, RUBBLE OR OTHER CONSTRUCTION DEBRIS ENCOUNTERED DURING THE WORK.
- 2. KEEP EXCAVATION FREE OF WATER BY BAILING, PUMPING OR A SYSTEM OF DRAINAGE AS REQUIRED AND PROVIDE PUMPS, SUCTION AND DISCHARGE LINES OR WELL POINTS OF SUFFICIENT CAPACITY AND MAINTAIN UNTIL SUCH TIME AS THE PERMANENT DRAINAGE SYSTEM IS INSTALLED OR UNTIL THE CONSULTANT'S APPROVAL OF REMOVAL OF EQUIPMENT IS OBTAINED. TAKE ALL NECESSARY MEASURES TO PREVENT FLOW OF WATER INTO THE EXCAVATION.
- 3. PROTECT THE BOTTOM AND SIDES OF EXCAVATED PITS AND TRENCHES FROM FREEZING. PROTECT ALSO FROM EXPOSURE TO THE SUN AND WET WEATHER TO PREVENT CAVE-INS AND SOFTENING OF THE BED UPON WHICH CONCRETE OR DRAINS REST.
- 4. EXCAVATIONS MUST NOT INTERFERE WITH THE NORMAL 45 DEGREE PLANE OF BEARING FROM THE BOTTOM OF ANY FOOTING.
- 5. KEEP BOTTOMS OF EXCAVATIONS CLEAN AND CLEAR OF LOOSE MATERIALS LEVELLED AND STEPPED AT CHANGES OF LEVELS WITH EXCEPTION OF EXCAVATIONS MADE FOR DRAINAGE PURPOSES AND THOSE SLOPE AS REQUIRED.
- 6. IF THE EXCAVATIONS REVEAL SEEPAGE ZONES, SPRINGS OR OTHER UNEXPECTED SUB-SURFACE CONDITIONS WHICH MAY NECESSITATE REVISIONS OR ADDITIONS TO ANY DRAINAGE SYSTEM, INFORM THE CONSULTANT IMMEDIATELY SO THAT REMEDIAL CAN BE

#### BACK FILLING

- 1. PROCEED PROMPTLY WITH BACKFILLING AS THE BUILDING REPAIRS PROGRESSES, AND AS WORK TO BE BACKFILLED HAS BEEN INSPECTED AND APPROVED BY THE CONSULTANT. THE BACKFILL SHOULD BE PLACED IN LIFTS NOT GREATER THAN 8" THICK IN THE LOOSE STATE, EACH LIFT BEING COMPACTED WITH A SUITABLE COMPACTOR TO THE SPECIFIED DENSITY.
- 2. DO NOT COMMENCE BACKFILLING OPERATIONS UNTIL SITE DRAINAGE SYSTEMS AND WATERPROOFING HAS BEEN INSPECTED AND APPROVED BY CONSULTANT.
- 3. WITHDRAW SHORING MATERIAL DURING BACKFILL. LUMBER LEFT IN PLACE WITHOUT THE

CONSULTANT'S APPROVAL WILL NOT BE PAID FOR BY THE OWNER.

- 4. WHERE FILL IN PLACED ADJACENT TO STRUCTURES OR VULNERABLE BUILDING COMPONENTS OR IN RESTRICTED AREAS, THE FILL SHALL BE COMPACTED TO THE SAME DEGREE AS SPECIFIED BY SUITABLE EQUIPMENT APPROVED BY THE CONSULTANT. AVOID DAMAGE TO OR DISPLACEMENT OF EXISTING/NEW STRUCTURES.
- 5. ADD WATER IN AMOUNTS REQUIRED ONLY TO ACHIEVE THE OPTIMUM MOISTURE CONTENT, IN ACCORDANCE WITH ASTM D1557.
- 6. BACKFILL SHALL BE FREE OF SNOW AND ICE, TOPSOIL, CONSTRUCTION DEBRIS AND OVERSIZED BOULDERS GREATER THAN 150mm

#### **FOUNDATIONS**

- 1. ALL FOOTINGS SHALL BEAR ON UNDISTURBED NATIVE SOIL APPROVED BY THE GEOTECHNICAL CONSULTANTS. REPORT ANY DOUBTFUL BEARING CONDITIONS TO THE STRUCTURAL ENGINEER BEFORE PLACING FOOTINGS. REFER TO SOILS REPORT FOR EXCAVATION. BACKFILLING AND DEWATERING PROCEDURES.
- 2. FROST COVER: BOTTOM OF FOUNDATION WALL AND GRADE BEAMS TO HAVE A MINIMUM
- 5'-0" COVER TO FINISHED EXTERIOR GRADE FOR FROST PROTECTION UNLESS NOTED. 3. MATERIALS FOR BACKFILL SHALL BE GRAN 'A' AND GRAN 'B' CONFORMING TO OPSS
- 4. ALL PIERS AND FOOTINGS UNDER COLUMNS TO BE CENTERED UNDER COLUMNS UNLESS NOTED OTHERWISE ON THE PLAN.

STANDARDS COMPACTED TO 98% STANDARD PROCTOR MAX. DRY DENSITY.

5. FOUNDATION DESIGN BASED ON ASSUMED BEARING CAPACITY OF 150 kPg (SLS) AND 225 kPa (ULS) ON NATIVE MATERIAL. THE BEARING CAPACITY MUST BE CONFIRMED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING FOUNDATIONS.

#### CONCRETE

1. CONCRETE CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF CAN/CSA-A23.1 AND CAN/CSA-A23.3 WITH THE FOLLOWING PROVISION:

LOCATION	DESIGN STRENGTH (28 DAYS)	SLUMP	EXPOSURE CLASS
EXTERIOR FOOTINGS/ FOUNDATION WALLS AND PIERS	25 MPa	80± 30	F-2
INTERIOR FOOTINGS/FOOTING WALLS AND PIERS	25 MPa	80± 30	N
SLAB ON GRADE	25 MPa	80± 30	N*
ALL OTHER INTERIOR CONCRETE	25 MPa	80± 30	N
ALL EXTERIOR REINFORCED CONCRETE (STAIRS & RETAINING WALLS)	35 MPa U.N.O.	80± 30	C-1
EXTERIOR UN-REINFORCED CONCRETE (CURBS & CONCRETE WALKWAYS)	35 MPa U.N.O.	80± 30	C-2

- 2. NO ADDITIONAL WATER SHALL BE ADDED AT THE JOB SITE. CONCRETE WHICH HAS BEEN WATERED OR DOES NOT MEET SPECIFICATIONS SHALL BE REJECTED.
- DURING WINTER WEATHER BELOW 5 °C PROVIDE TEMPORARY HEATING OF CONCRETE
- ACCORDANCE WITH THE REQUIREMENTS OF CSA A23.1.
- WHEN PIPES, CONDUITS, OR SLEEVES ARE REQUIRED TO PENETRATE CONCRETE
- ASSEMBLIES, THE FOLLOWING SHALL BE OBSERVED: SPACING OF SUCH ITEMS SHALL BE 3 DIAMETERS ON CENTER. - CONCRETE SHALL NOT BE PENETRATED WITHIN 600mm (24") OF
- CONCENTRATED LOADS. - ANY PENETRATION SHALL BE A MINIMUM OF 600mm (24") FROM ALL EDGES
- SUCH AS ENDS AND TOPS OF WALLS. FOR ANY PENETRATIONS GREATER THAN 300mm (12"), CONSULT THE ENGINEER FOR REVIEW AND DETERMINATION OF EXTRA REINFORCEMENT REQUIREMENTS IF APPLICABLE.

#### **SLAB ON GRADE**

- 1. CAST SLAB ON GRADE ON 8" MIN CRUSHED STONE AND COMPACTED SUB-GRADE UNLESS NOTED OTHERWISE.
- 2. REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL REQUIREMENTS OF COMPOSITION OF MATERIALS BELOW GRADE.
- 3. SAWCUT WITHIN 6 TO 18 HOURS. REFER TO THE DRAWINGS FOR SAWCUT
- 4. DO NOT CAST SLAB MORE THAN 30 METERS IN LENGTH IN FITHER DIRECTION.
- PLACE SLAB IN STRIP PATTERN. KEY CONSTRUCTION JOINTS AS DETAILED 5. MAINTAIN MINIMUM SPECIFIED THICKNESS AT ALL DEPRESSIONS AND CHANGES IN ELEVATIONS.
- 6. REFER TO ARCHITECTURAL DRAWINGS FOR EXTENT AND LOCATION OF ALL FINISHES, DEPRESSIONS AND SLOPES.
- 7. WELDED WIRE MESH REINFORCING IN SLABS ON GRADE MUST BE PROPERLY

# CONCRETE REINFORCEMENT

1. THE CLEAR DISTANCE BETWEEN REINFORCING STEEL AND SURFACE OF CONCRETE SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE.

CHAIRED. LIFTING OF THE WIRE MESH DURING POURS WILL NOT BE ACCEPTED.

LOCATION	CLEAR COVER
FOOTINGS	75mm(3") UNDERSIDE 50mm(2") TOP AND ENDS
WALLS	50mm(2") AGAINST EARTH (20M BAR OR GREATER) 40mm(1½") AGAINST EARTH (15M BAR) 40mm (1½") AGAINST FORM (20M BAR OR GREATER) 25mm (1") AGAINST FORM (15M BAR)
SLABS	25mm(1") TOP BARS 25mm(1") BOTTOM BARS
COLUMNS/PIERS	40mm(1½") TO TIES
SURFACE IN CONTACT WITH GROUND	75mm(3")

- 2. STRUCTURAL GROUT SHALL BE NON-SHRINK, NON METALLIC M-BED STANDARD PREMIX BY SIKA OR APPROVED EQUAL.
- 3. DETAIL REINFORCING STEEL IN ACCORDANCE WITH "REINFORCING STEEL MANUAL OF STANDARD PRACTICE" LATEST EDITION.
- REINFORCING BAR SPLICES FOR DEFORMED BARS: COLUMNS — COMPRESSION LAP UNLESS NOTED WALLS - CLASS 'B' TENSION SPLICE UNLESS NOTED
- ALL OTHERS CLASS 'B' TENSION LAP UNLESS NOTED 5. ALL REINFORCING STEEL SHALL BE DEFORMED HARD GRADE BILLET STEEL
- CONFORMING TO CSA G30.18 GRADE 400. 6. WELDED STEEL WIRE FABRIC, PLAIN TYPE CONFORMING TO ASTM A1064/
- A1064M-18a IN FLAT SHEETS NOT ROLLED. 7. ALL CONCRETE REINFORCEMENT MUST BE PROPERLY CHAIRED WITH APPROVED BAR SUPPORTS.
- 8. PROVIDE CHAIRS, SPACER BARS, SUPPORT BARS AND OTHER ACCESSORIES TO SUPPORT REINFORCING IN ACCORDANCE WITH THE LATEST EDITIONS OF CSA A23.1 AND CSA A23.3. CHAIRS TO BE PLASTIC, PLASTIC TIPPED OR CONCRETE. ALL TIE WIRE. CHAIRS AND BAR SUPPORTS USED FOR COATED REINFORCING SHALL BE NON-METALLIC OR PROTECTED WITH ACCEPTABLE COATING.
- 9. CHAIRS SHALL BE SPACES AT 1200mm O.C. MAXIMUM.

#### **MASONRY**

CAN3-A23.1

- 1. MASONRY WORK IN ACCORDANCE WITH CAN/CSA A370 AND CAN/CSA A371 EXCEPT WHERE SPECIFIED OTHERWISE.
- 2. ONLY TYPE 'S' MORTAR SHALL BE USED, MINIMUM STRENGTH SHALL BE 12.4 MPa AT
- 3. GROUT (PEA GRAVEL) AT BOND BEAMS AND GROUTED HOLLOW BLOCKS TO BE A MINIMUM OF 20MPa COMPRESSIVE STRENGTH AT 28 DAYS. IN ACCORDANCE WITH
- 4. MORTAR FOR EXPOSED EXTERIOR MASONRY SHALL BE AIR ENTRAINED.
- 5. PROVIDE LATERAL RESTRAINT AT TOP OF NON-LOAD BEARING BLOCK PARTITIONS AS INDICATED ON TYPICAL DETAILS.
- 6. PROVIDE VERTICAL WALL REINFORCING FOR FULL HEIGHT OF LIFT, CONTINUOUS FROM FLOOR TO FLOOR/ROOF, WITH CLASS B LAPS.
- 7. MASONRY WORK SHALL CONFORM TO CAN3-S304 AND ITS REFERENCED DOCUMENTS,
- A. CONCRETE BLOCK TO CSA-A165.1, TYPE H15/A, SEE ARCH. DWGS. FOR
- FIREWALL REQUIREMENTS UNLESS NOTED OTHERWISE. (BASED ON NET AREA) -H15/C FOR FIREWALL.
- MORTAR TO CSA-A179M, TYPE 'S' FOR ALL WALLS. GROUT TO CSA-A179-M. MASONRY WIRE REINFORCING TO CSA-G30.14.
- REINFORCING BARS TO CSA-G30.18-M-400 MPa. WELDED REINFORCING BARS TO CSA-G30.18-400 MPa.
- H. PRACTICE TO CAN/CSA A371. 8. STRUCTURAL DRAWINGS INDICATE ONLY LOAD-BEARING WALLS

CONNECTION TO CAN/CSA A370.

BEEN INSPECTED

- 9. SUBMIT EVIDENCE OF MORTAR AND GROUT STRENGTH. FIELD CONTROL AND TESTING SHALL COMPLY WITH REQUIREMENTS OF CLAUSE 5 OF CAN3-304.
- 10. PROVIDE TEMPORARY BRACING OF MASONRY WORK UNTIL PERMANENT LATERAL SUPPORT IS IN PLACE.
- 11. PROVIDE LINTELS OVER ALL OPENINGS IN MASONRY WALLS. SEE LINTEL SCHEDULE FOR REQUIREMENTS.
- 12. REFER TO TYPICAL DETAILS FOR BOND BEAM AND BEARING REQUIREMENTS AT FLOORS AND ROOFS.
- 13. MINIMUM STANDARD LAP LENGTH: WIRE REINF. 200mm (8") 10M BARS - 400mm (16") 15M BARS - 600mm (24") 20M BARS - 800mm (32")
- 14. UNLESS NOTED OTHERWISE, PROVIDE 2-15M VERTICAL BARS FULL HEIGHT AT: A. UNSUPPORTED ENDS OF WALLS B. EACH SIDE OF CONTROL JOINTS
- 15. PROVIDE CLEANOUT PORT AT BOTTOM OF EACH GROUTED CORE WHEN REQUIRED BY ENGINEER. DO NOT CLOSE PORT OR PLACE GROUT UNTIL CORE AND STEEL HAVE
- 16. FILL CELLS CONTAINING VERTICAL REINFORCING AND BOLTS WITH GROUT VIBRATE OR PUDDLE TO FILL CELLS COMPLETELY.
- 17. FILL CELLS IN 1500mm (60") LIFTS MAXIMUM OR BETWEEN BOND BEAMS, WHICHEVER IS LESS, UNLESS SPECIAL PROVISIONS ARE MADE TO ENSURE FULL GROUT COLUMNS HAVE BEEN MADE TO THE SATISFACTION OF THE ENGINEER.

18. CONTROL JOINTS SHALL BE INSTALLED AT MAXIMUM SPACING OF 6000mm (20'-0"), IF

- NOT OTHERWISE SHOWN ON ARCHITECTURAL DRAWINGS. REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION. 19. FILL BLOCK CORES UNDER ALL BEAMS, JOISTS AND OTHER CONCENTRATED POINT
- LOADS WITH CONCRETE GROUT. GROUT SHALL EXTEND A MINIMUM OF 600mm (24") BELOW LOAD 20. CONTROL JOINTS AND EXPANSION JOINTS SHALL BE CONTINUED THROUGH BOND
- BEAMS IF NOT OTHERWISE SHOWN. 21. NO MASONRY WORK SHALL BE PERMITTED WITH TEMPERATURE BELOW 5° CELSIUS,

UNLESS PROVISIONS ARE MADE FOR HEATING THE MATERIALS AND PROTECTING THE

22. SET BASE PLATES ON MASONRY ON MIN. 25MPg NON-SHRINK GROUT FOR LEVELING.

23. FIRST COURSE OF MASONRY TO BE LAID IN A FULL BED OF MORTAR. ALL OTHER

COURSES TO BE LAID W	WITH MOR	TAR AT FACE SHELL BED A	ND HEAD JOINTS.
CONCRETE BLOCK	K MASO	NRY COMPRESSIVE ST	RENGTH (MPa)
LOCATION	NET	GROSS (f'm) FOR HOLLOW BLOCK	GROSS (f'm) FOR SOLID OR GROUTED BLOCK

BLOCK 9.8 7.5 24. POCKETS FOR STEEL BEAMS AND JOISTS SHALL BE GROUTED SOLID AND THE WALL MADE GOOD AFTER

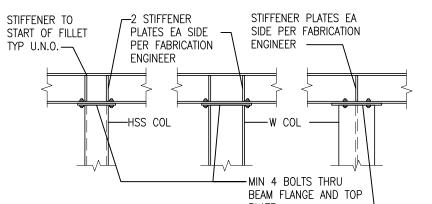
# STRUCTURAL STEEL

TYP. ALL CONCRETE

- 1. ALL STRUCTURAL STEEL HSS AND W SECTIONS TO BE G40.21M-350W CLASS C. ALL OTHERS TO BE G40.21M-300W.
- 2. DESIGN FORCES INDICATED ON DRAWINGS FOR STRUCTURAL STEEL WORK ARE UN-FACTORED FORCES UNLESS NOTED OTHERWISE.

3. ALL CONNECTIONS TO BE DESIGNED BY FABRICATOR UNLESS NOTED OTHERWISE. ALL

- BEAM CONNECTIONS TO BE STANDARD SHEAR CONNECTIONS IN COMPLIANCE WITH CISC, UNLESS NOTED OTHERWISE 4. FABRICATOR'S ENGINEER MUST BE LICENSED IN THE PROVINCE OF ONTARIO.
- 5. ALL ERECTION BOLTS SHALL BE ASTM GRADE A325 MINIMUM, AND SHALL BE DESIGNED BY STEEL FABRICATOR'S ENGINEER FOR TRANSFER OF ALL LOADS.
- 6. BOLTED CONNECTIONS SHALL HAVE A MINIMUM OF TWO BOLTS IN EACH CONNECTION PIECE AND DESIGNED AS BEARING CONNECTIONS U.N.O.
- 7. UNLESS NOTED OTHERWISE, WHERE BEAMS FRAME OVER COLUMNS, PROVIDE FULL HEIGHT, FULL WIDTH STIFFENER PLATES ON EACH SIDE OVER COLUMN



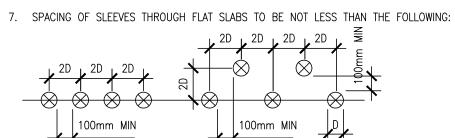
- 9. FABRICATOR'S ENGINEER SHALL DESIGN TOP PLATES AND THEIR CONNECTIONS TO FULLY TRANSFER VERTICAL AND HORIZONTAL LOADS AS WELL AS MOMENTS WHEN REQUIRED PER CISC STANDARDS.
- 10. FABRICATION, ERECTION AND WORKMANSHIP SHALL CONFORM TO CSA S16.1.
- 11. ALL WELDING SHALL CONFORM TO CSA W59 AND SHALL BE PERFORMED BY A WELDER QUALIFIED UNDER CSA W47.
- 12. ALL STRUCTURAL STEEL SHALL BE PAINTED WITH ONE SHOP APPLIED COAT OF PRIMER. SPOT PRIME ALL WELDED AREAS.
- 13. REMOVE PAINT FILM FROM ALL STEEL SURFACES TO BE WELDED. SPOT PRIME AS
- REQUIRED. 14. ALL WELDED CONNECTIONS SHALL BE WITH CSA W48 SERIES ELECTRODES.
- 15. PROVIDE DRAINAGE HOLES TO ARCHITECT'S APPROVAL IN ALL EXPOSED EXTERIOR HSS FRAMING.
- 16. PROVIDE CAP PLATES ON ALL HSS FRAMING MEMBERS UNLESS NOTED OTHERWISE.
- 17. DO NOT CUT OR CORE ANY OPENINGS IN ANY STRUCTURAL STEEL MEMBERS WITHOUT PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER.
- 18. WHERE A STRUCTURAL STEEL SHAPE SHOWN ON THE DRAWINGS IS UNAVAILABLE, A SHAPE OF EQUAL OR GREATER SECTION PROPERTIES AND STRUCTURAL CAPACITY SHALL BE SUBSTITUTED, UPON APPROVAL BY OWNER AND CONSULTANT AT NO
- 19. ALL EXPOSED STEEL SHALL BE HOT DIP GALVANIZED.

#### CONDUITS, PIPES AND SLEEVES EMBEDDED IN CONCRETE

- EXCEPT WHEN APPROVED BY THE STRUCTURAL ENGINEER, PIPES, CONDUITS, AND SLEEVES EMBEDDED IN CONCRETE SHALL BE INSTALLED IN ACCORDANCE WITH THE FOLLOWING
- A. NOT WITHSTANDING THE SATISFYING OF THESE GUIDELINES. THE CONDUIT, SLEEVES. PIPES. ETC. SHALL NOT IMPAIR THE STRUCTURAL STRENGTH AND SHALL BE MOVED IF
- SO DIRECTED BY THE STRUCTURAL ENGINEER. B. CENTRELINE SPACING TO BE NOT LESS THAN 3 DIAMETERS - UNLESS NOTED
- C. CENTRELINE SPACING BETWEEN PARALLEL CONDUIT AND REINFORCING BARS TO BE 3 DIAMETERS - UNLESS NOTED OTHERWISE

#### CONDUITS, PIPES AND SLEEVES EMBEDDED IN CONC. CONT'D

- D. ADD REINFORCING AT POINTS OF CONGESTION AS DIRECTED BY STRUCTURAL ENGINEER.
- 2. FOR SLABS CONDUITS IN THE PLANE OF THE SLAB.
- A. LOCATE BETWEEN TOP AND BOTTOM REINFORCING. B. MAXIMUM SIZE IN ONE LAYER TO BE NOT MORE THAN 1/4 CONCRETE THICKNESS.
- C. THREE LAYERS OR MORE CROSSING WILL NOT BE PERMITTED.
- 3. FOR COLUMNS ELECTRICAL BOXES, CONDUIT, SLEEVES OR EMBEDDED PIPES ARE NOT ALLOWED WITHOUT THE WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.
- 4. FOR BEAMS THE TOTAL MAXIMUM SIZE OF HORIZONTAL CONDUIT PARALLEL TO THE BFAM NOT TO EXCEED 4% OF THE AREA. NO SLEEVES THRU ANY BEAMS OR SLAB BANDS UNLESS APPROVED IN WRITING BY THE STRUCTURAL ENGINEER.
- 5. FOR SHEAR WALLS ELECTRICAL BOXES, CONDUIT, SLEEVES OR EMBEDDED PIPES
- ARE NOT ALLOWED WITHOUT THE WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER. 6. FOR NON-SHEAR WALLS - CONDUIT, SLEEVES OR EMBEDDED PIPES:
- NO HORIZONIAL RUNS PERMITTEL VERTICAL RUNS TO HAVE MINIMUM 50mm CONCRETE COVER AND SHALL HAVE A MINIMUM CLEAR SPACING OF 4 DIAMETERS.



- SLEEVES IN FLAT SLABS NOT TO BE LOCATED NEXT TO COLUMNS UNLESS APPROVED BY ENGINEER IN WRITING.
- CLEAR DIMENSION FOR SLEEVES TO COLUMNS TO BE 1200mm MIN. UNLESS NOTED OTHERWISE ON STRUCTURAL DRAWINGS.

#### TEMPORARY WORKS

MAX DIAMETER = 1/4 WALL THICKNESS

- 1. THE CONTRACTOR SHALL DESIGN, PROVIDE, ERECT, MAINTAIN REMOVE AND ASSUME FULL AND SOLE RESPONSIBILITY FOR ALL TEMPORARY WORKS REQUIRED FOR THE SAFE AND COMPLETE EXECUTION OF THE WORKS.
- CONTRACT, THE CONTRACTOR SHALL MAKE ADEQUATE PROVISION FOR ALL LIKELY CONSTRUCTION LOADING AND PROVIDE SUFFICIENT BRACING AND PROPS TO KEEP THE WORKS IN PLUMB AND ALIGNMENT AND FREE FROM EXCESSIVE DEFLECTION. 3. ACCESS OF HEAVY CONSTRUCTION EQUIPMENT AND ACCUMULATION OF CONSTRUCTION

MATERIALS ON THE FLOORS ARE NOT PERMITTED, UNLESS SUCH HAVE BEEN CATERED

FOR IN THE CONTRACTOR'S TEMPORARY WORK DESIGN TO THE SATISFACTION OF THE

2. IN THE EXECUTION OF THE TEMPORARY WORKS AND FOR THE DURATION OF THE

4. SUBMIT SHOP DRAWINGS FOR ALL TEMPORARY WORKS FOR REVIEW BEFORE FABRICATION COMMENCES. SHOP DRAWINGS SHALL BE SEALED BY A PROFESSIONAL

# **INSPECTIONS AND TESTING**

ENGINEER REGISTERED IN THE PROVINCE OF ONTARIO.

1. THE FOLLOWING ITEMS SHALL BE INSPECTED OR TESTED BY INDEPENDENT INSPECTION/ TESTING AGENCIES DESIGNATED BY THE CLIENT. MATERIALS AND WORKMANSHIP NOT CONFORMING TO THE SPECIFICATIONS SHALL BE REJECTED BY THE CONTRACTOR. REPORTS AND TEST RESULTS SHALL BE PROMPTLY SUBMITTED TO THE ENGINEER FOR REVIEW. TESTING SHALL INCLUDE BUT NOT BE LIMITED TO:

- 1. CONCRETE AND GROUT TESTING IN ACCORDANCE WITH CSA A23.2 LATEST EDITION AND THE SPECIFICATIONS. INCLUDING THE REQUIREMENTS OF SLUMP. AIR AND AGE PRIOR TO BEING USED. CONTRACTOR TO KEEP RECORDS OF POUR DATES, TESTING PERFORMED, CLASS OF CONCRETE USED AND TEST RESULTS FOR ALL ITEMS POURED.
- TESTING TO DETERMINE THE IN-SITU STRENGTH OF CONCRETE FOR EARLY FORM REMOVAL PURPOSES WITH THE TYPE OF TEST BEING DETERMINED ON THE ADVICE OF THE TESTING AGENCY, REPAIRS NECESSARY TO THE STRUCTURE AS A RESULT OF THESE TESTS SHALL BE MADE BY THE CONTRACTOR AT NO EXPENSE TO THE OWNER.
- 3. APPROVAL OF CONCRETE AND MORTAR MIX DESIGNS.
- 4. TESTING AS REQUIRED AND SPECIFIED BY THE ENGINEER TO DETERMINE THE IN-SITU STRENGTH OF CONCRETE WHICH FAILS TO MEET THE SPECIFIED REQUIREMENTS OR WHICH, DUE TO APPEARANCE, DAMAGE OR DEFECTS MAY BE DEEMED REJECTABLE. CORES SHALL BE ACQUIRED AND TESTED BY THE DESIGNATED TESTING AGENCY BUT ANY REPAIRS NECESSARY TO THE STRUCTURE AS A RESULT OF THESE TESTS SHALL BE PERFORMED AT NO COST TO THE OWNER.

STEEL MOMENT CONNECTION

# $\bigcirc$

FROM FLOOR ELEVATION

DENOTES COMPOSITE STEEL

ANCHOR BOLT INDICATES TENSION. AI TERNATE ARCHITECTURAL BOTTOM CAMBER IN 'x' mm COLUMN BRACE CENTRE TO CENTRE

2. REVIEW OF DRAWINGS APPLIES TO GENERAL ARRANGEMENT ONLY FOR THE PURPOSE OF ASCERTAINING CONFORMANCE WITH THE GENERAL DESIGN CONCEPT. THIS REVIEW DRAWINGS NOR DOES IT RELIEVE THE CONTRACTOR OF HIS RESPONSIBILITY FOR MAKING THE WORK COMPLETE. ACCURATE AND IN ACCORDANCE WITH THE STRUCTURAL CONTROL JOINT CONNECTION CONTINUOUS

**CENTRELINE** COLUMN : CONC CONT CONCRETE CONTINUOUS DIAGONAL BRACE

DIAGONAL DIMENSION DOWN DRAWIN

PROFESSIONAL ENGINEER 3. SUBMIT FOR REVIEW ALL PROPOSED CONCRETE MIX DESIGNS. SUBMIT AT LEAST 15 WORKING DAYS PRIOR TO START OF WORK.

#### AND LAYOUT DRAWINGS OF CONCRETE ISOLATION AND HOUSEKEEPING PADS. 5. REGULARLY SUBMIT REPORTS OF ALL CONCRETE TESTING AS SOON AFTER TESTING IS

PERFORMED AS POSSIBLE

REINFORCED CONCRETE

ENGINEERED MASONRY TEST

STRUCTURAL STEEL

REINFORCING STEEL

1. TESTS TO VERIFY STRENGTH OF GROUT AND MORTAR

1. VISUAL INSPECTION OF ALL WELDS, TORQUE TESTING OF BOLTED CONNECTIONS AND

CHECK ON BEARING, PLUMBNESS AND ALIGNMENT OF STEEL STRUCTURES.

2. NON-DESTRUCTIVE TESTING TO VERIFY THE QUALITY OF WELDING, WHERE DEEMED

1. CONTRACTOR SHALL ADVISE BARRY BRYAN ASSOCIATES (1991)LTD. OF REQUIRED

SHOP DRAWINGS AND SUBMITTALS

REINFORCING STEEL INSPECTIONS AT LEAST 24 HOURS PRIOR TO CLOSING OF

COLUMN OR WALL FORMS AND 24 HOURS PRIOR TO PLACING OF CONCRETE IN

1. REPRODUCTIONS OF THE STRUCTURAL DRAWINGS SHALL NOT BE ACCEPTED AS SHOP

DOFS NOT IMPLY APPROVAL OF DETAIL DESIGN OR QUANTITIES IN SUBMITTED.

4. SHOP DRAWING SUBMITTALS SHALL ALSO INCLUDE STAIR DETAILS, GUARDRAILS, WITH

ENGINEER IN ACCORDANCE WITH N.B.C. SECTIONS 2.3.4.4 AND 2.3.4.5 LATEST

1. SUBMIT FOR REVIEW 2 COPIES AND 1 SEPIA OF REINFORCEMENT PLACING DRAWINGS

PROVIDE DETAILS OF DESIGN AND CONSTRUCTION OF FORMS AND FALSEWORK,

SHORING AND RESHORING AND ANY SPECIAL REQUIREMENTS FOR STRIPPING OF

AND BARLISTS FOR EVERY PORTION OF THE STRUCTURE. SHOW WALLS IN FULL

ELEVATION. SHOW SLABS WITH REINFORCING STEEL CALLED UP DIRECTLY ON PLAN.

FORMWORK. ALL SUCH DESIGN SHEETS SHALL BE PREPARED AND STAMPED BY A

GUARD RAIL LOADING WHICH SHALL BE SIGNED AND STAMPED BY A PROFESSIONAL

REGISTERED AND LICENSED TO PRACTICE IN THE PROVINCE OF ONTARIO.

DRAWINGS. ALLOW 10 WORKING DAYS FOR SHOP DRAWING REVIEW.

3. DO NOT FABRICATE MATERIALS BASED ON REJECTED OR DISAPPROVED SHOP

DRAWINGS. "PROFESSIONAL ENGINEER" IN THE FOLLOWING PARAGRAPHS SHALL BE

QUESTIONABLE BY VISIBLE DEFECTS OR WHERE REQUIRED BY THE ENGINEER.

1. SUBMIT EVIDENCE OF MORTAR AND GROUT STRENGTH.

STRUCTURAL STEEL AND STEEL DECK

1. SUBMIT WITH SHOP DRAWINGS: DECKING PLAN, PROFILE, DIMENSIONS, CORE THICKNESS, CONNECTIONS TO SUPPORTS, REQUIRED BEARINGS, CLOSURES AND

STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER. SHOP DRAWING SUBMITTALS SHALL INCLUDE STEEL BEAM CONNECTIONS ALONG WITH DESIGN CALCULATIONS. JOIST CALCULATIONS SHALL BE SIGNED AND STAMPED BY A

AS-BUILT DETAILS OF ALL ASPECTS OF THE STRUCTURE, FOR REVIEW DURING

CONSTRUCTION AND FOR SUBMISSION AT THE END OF THE PROJECT.

SUBMIT FOR REVIEW DIGITAL COPIES OF ERECTION DRAWINGS FOR ALL STRUCTURAL

STEEL ELEMENTS. ALL CONNECTIONS SHALL BE DESIGNED AND THE DRAWINGS

DO NOT SCALE THE DRAWINGS

AND SEALED BY THE CONSULTANT

NO. ISSUES

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NO. REVISIONS

DATE

LONG LEG HORIZONTAL LONG LEG VERTICAL MOMENT CONNECTION TO HAVE PROJECT: FACTORED MOMENT

> - former Lakefield Post Office 12 QUEEN STREET,

TOWNSHIP OF SELWYN



**BARRY BRYAN ASSOCIATES** DRAWN BY: Architects KM/CE

CHECKED BY: DB MAY 2022 Tel: (905) 666-5252

PROJECT NO:

**S101** 

July 15, 2022

% COMPLETE:

**ABBREVIATIONS** 

DISTANCE TO TOP OF BEAM OR FOOTING FROM DATUM ELEVATION OF FLOOR OR

DISTANCE TO TOP OF SLAB

—— DIRECTION OF SLOPE

SHORING LOAD IN kN FACTORED AXIAL LOAD (kN). INDICATES COMPRESSION ABOVE FINISHED FLOOR

FACTORED COMPRESSION (kN)

COMPLETE WITH (INCLUDING) DFTAIL DIAMETER

DEAD LOAD (kN/m2) DOWEL FPOXY COATED

REINFORCEMEN

FACH SIDE

EACH WAY

FINISHED

FLOOR

**FXISTING** 

EACH END FACH FACE **ELECTRICAL** ELEVATION 4. SUBMIT FOR REVIEW DRAWINGS OF ALL PROPOSED CONSTRUCTION JOINTS LOCATIONS, FQUAI

> EXPANSION JOIN FXTFRIOR FLOOR DRAIN FOUNDATION FAR FACE

**FOOTING** GALVANIZED GROUND HIGH BEAM HOLLOW CORE CONCRETE

HEAVY DUTY MASONRY REINFORCEMENT HOOKED REINFORCEMENT HORIZONTAL

HORIZONTAL SLOTTED

PROFESSIONAL ENGINEER IN ACCORDANCE WITH N.B.C. SECTIONS 2.3.4.4 AND 2.3.4.5. HOLLOW STRUCTURAL SECTION HORIZONTAL AND VERTICAL INSIDE FACE 1. GENERAL CONTRACTOR SHALL MAINTAIN TWO SETS OF RECORD DRAWINGS WHICH SHOW INCLUDING

MAX

MCx

MECH

MEZZ

OWSJ

REINF

STIFF

TOS

U/S

STEEL ANGLE LOW BEAM LONG LIVE LOAD (kN/m2) LOWER LAYER

INTERIOR

MAXIMUM

NFAR FACE

PROJECTION

ROOF DRAIN

REVISION

STANDARD

STIFFENER

STRUCTURAL

**THICKNESS** 

TIE JOIST

TOP OF STEEL

UPPER LAYER

VERTICAL BRACE

VERTICAL SLOTTED

FACTORED WEIGHT

WELDED WIDE FLANGF

WELDED WIRE MESH

WALL PLATE

UNDERSIDE

TOP OF

REINFORCING

ROUGH OPENING

SUPERIMPOSED DL (EXCLUDING

STANDARD DUTY MASONRY

STEP DOWN FOOTING

RFINFORCEMENT

FACTORED TENSION FORCE

UNLESS NOTED OTHERWISE

FACTORED SHEAR FORCE

CONNECTION

SELF WEIGHT) (kN/m2)

PRECAST CONCRETE

CAPACIT IN 'x' kNm MECHANICAL **Accessibility Renovations** MEZZANINE MISCELLANEOUS FACTORED MOMENT (kN-m) FACTORED TORSION (kN-m) NOT IN CONTRACT

> LAKEFIELD ONTARIO NOT TO SCALE OUTSIDE FACE OPEN WEB STEEL JOIST POINT LOAD

DRAWING: **GENERAL NOTES** FACTORED VERTICAL REACTION

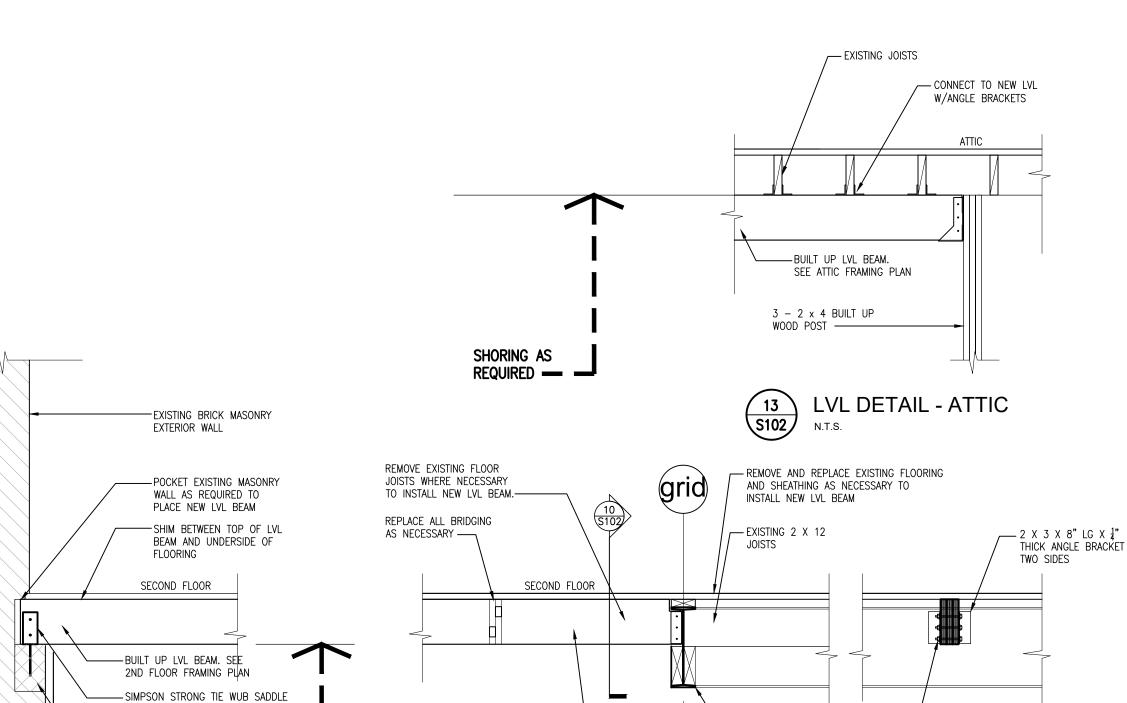


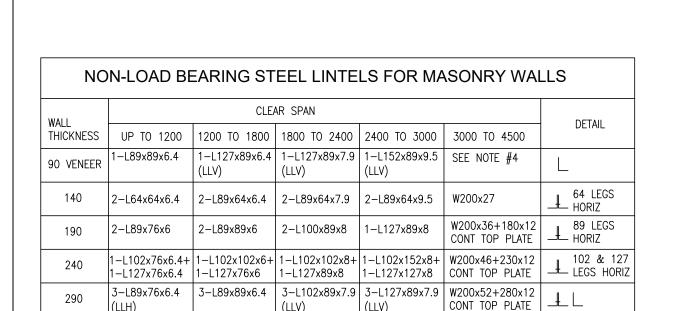
D. P. MCLAUGHLIN 100181883 DOC CONTROL: DESIGN BY RRA

Engineers Project Managers 250 Water Street Suite 201 Whitby Ontario L1N 0G5

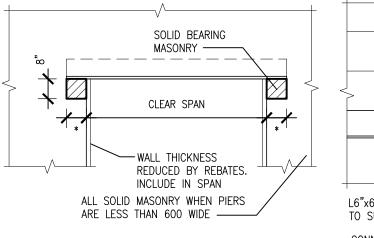
NTS Fax: (905) 666-5256 e-mail: bba@bba-archeng.com FILE:

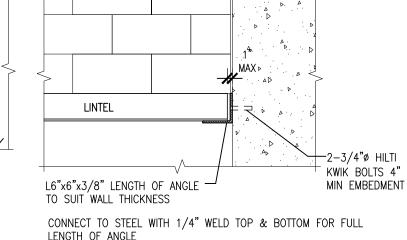
DRAWING NO:





- NOTES CONNECT ANGLES AT 600 C/C BY WELDING OR BOLTING FOR ANGLES WITH A TOTAL LENGTH OF 1800 OR MORE. USE 160 BOLTS OR 6x50 LONG WELDS.
- USE SCHEDULES FOR LINTELS OVER MECH OPENINGS IN ALL MASONRY WALLS UNLESS NOTED OTHERWISE ON PLAN. REFER TO MECH DWGS FOR LOCATIONS.
- OPENINGS TO BE LOCATED MIN 3 COURSES BELOW UNDERSIDE OF SLAB UNLESS APPROVED BY STRUCTURAL ENGINEER.
- FOR DOUBLE WYTHE WALLS PROVIDE 10mm GUSSETS @ 800 C/C STEEL LINTEL SUPPORT BLOCK ABOVE PLUS CONT. 320x10mm BOT PLATE.





BRACKET. DRILL AND EPOXY INTO

FILL VOID SOLID WITH CONCRETE

REQUIRED — —

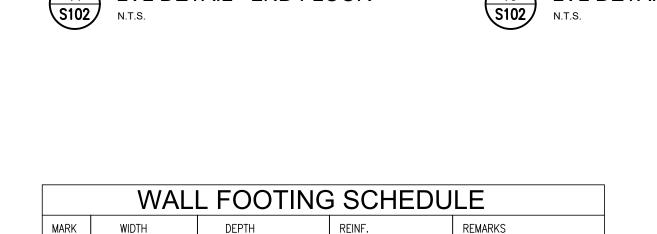
BLOCK BELOW SADDLE

LVL DETAIL - 2ND FLOOR

- \* BEARING LENGTH 150 (6") MINIMUM EACH END (STEEL ANGLE LINTEL) 200 (8") MINIMUM EACH END (BLOCK LINTELS)
- LENGTH OF ANGLE
- PROVIDE SHOP WELDED GALVANIZED STEEL STRAP ANCHORS AT 16" C/C FOR MASONRY ANCHORAGE TO STEEL COLUMN

### STRUCTURAL DRAWING LIST

- S101 GENERAL NOTES S102 SCHEDULES & TYPICAL DETAILS S201 FOUNDATION PLANS
- S202 GROUND FLOOR FRAMING PLANS
- S204 ATTIC FRAMING PLAN
- S203 SECOND FLOOR FRAMING PLANS S501 SECTIONS



—— BUILT UP LYL BEAM. SEE 2ND FLOOR FRAMING PLAN

11 LVL DETAIL - 2ND FLOOR

NOTES:

- EXISTING STEEL | 3 - §" BOLTS -

LVL DETAIL - 2ND FLOOR

BEAM

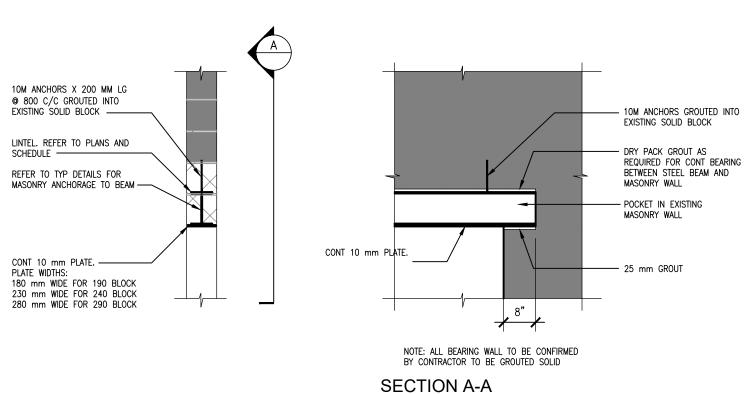
(3)-15M CONT. BOT.

	FOU	V NOITADN	VALL SCH	EDULE
MARK	THICKNESS	VERT. REINF.	HORIZ. REINF.	REMARKS
W1	8"	10M @ 16" C/C	10M @ 16" C/C	

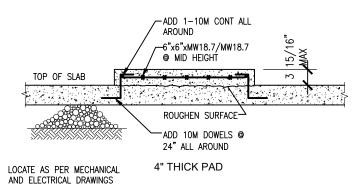
	BEARI	NG PLATE S	CHEDULE
MARK	SIZE	ANCHORAGE	REMARKS
BP1	7"x12"x5/8"	1-15M X 350LG + 1 1/2" HOOKED ANCHOR	
BP2	7"x12"x5/8"	5/8" DIA HILTI HY270 W 6" EMBEDMENT	

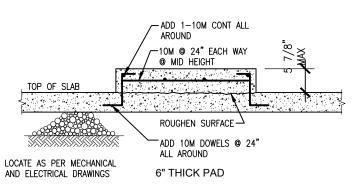
MASONRY WALL SCHEDULE					
MARK	DESCRIPTION	VERTICAL REINFORCEMENT	REMARKS		
MW1	8" BLOCK		15M VERT. REINF. @ 12" O/C w/ CORES FILLED SOLID w/ 2 COURSE BOND BEAM w/ 2-15M CONT. REINF. @ ALL FLOORS		
NOTES  1. ALL MASONRY WALLS ARE MW1 (TYP U.N.O.)  2. REFER TO GENERAL MASONRY NOTES FOR ADDITIONAL REINFORCING REQUIREMENTS					

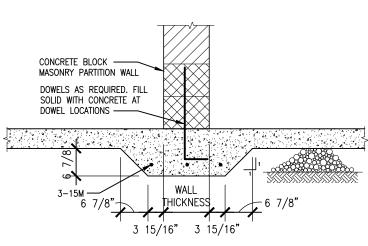
LINTEL SCHEDULE				
MARK	SIZE	DETAIL	REMARKS	
L1	8" x 16"		PROVIDE 15M TOP AND BOTTOM	
L2	W 200 X 31	9/\$102	WITH BOTTOM PLATE AND REBAR	



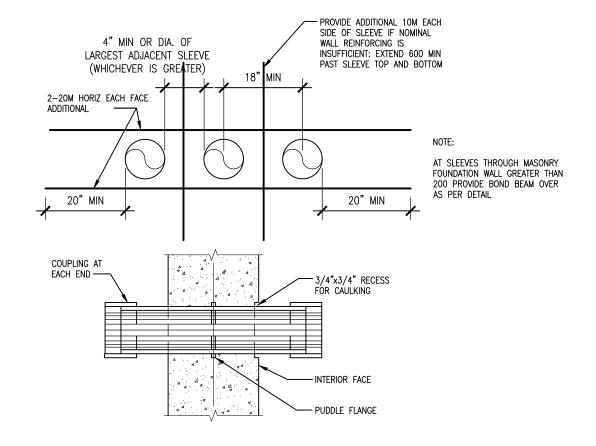




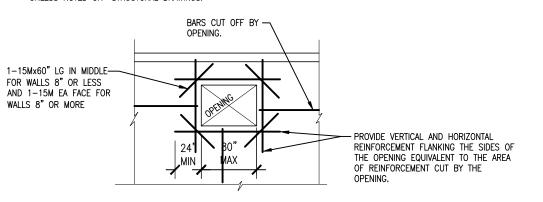




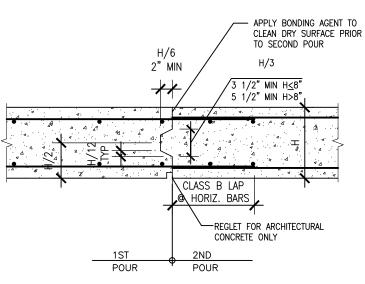




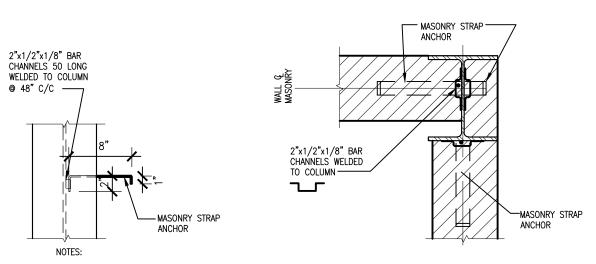
NOTES: 1. UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS, REINFORCE AROUND OPENING IN WALL AS PER DETAILS BELOW. 2. DO NOT MAKE OPENINGS LARGER THAN THE MAXIMUM DIMENSIONS NOTED BELOW WITHOUT PRIOR APPROVAL FROM THE ENGINEER OR UNLESS NOTED ON STRUCTURAL DRAWINGS.



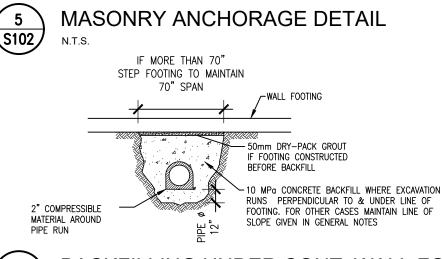
REINFORCING FOR OPENINGS IN CONCRETE WALLS <del>7</del> \$102



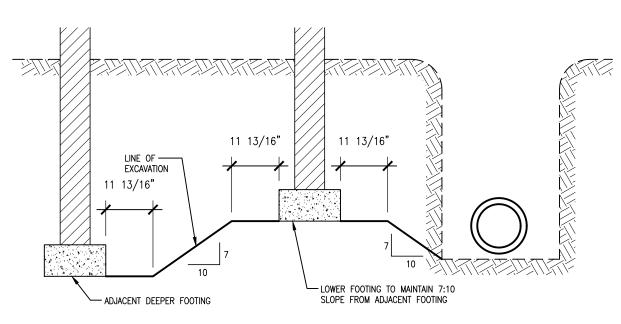
VERTICAL CONSTRUCTION JOINT DETAIL S102



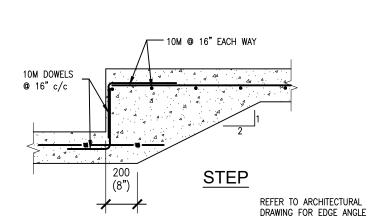
1. REFER TO SPECIFICATIONS FOR SPACING OF SLEEVE PLATES. 2. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF MASONRY.



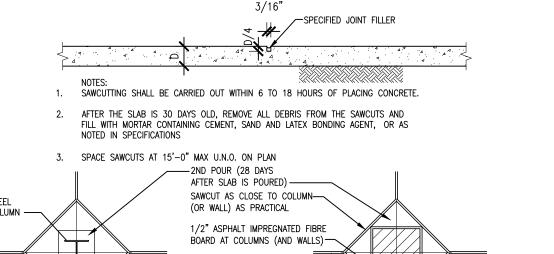
BACKFILLING UNDER CONT. WALL FOOTING



S102 SLOPE REQ'TS OF ADJACENT FOOTING



**SLAB ON GRADE STEPS** 



TYPICAL SLAB CONTROL JOINT DETAIL S102

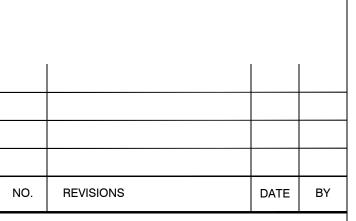
U.N.O. MAX AND AS CLOSE

TO COLUMN AS PRACTICAL —

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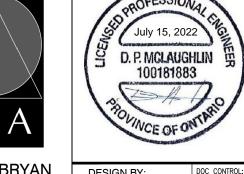
12 QUEEN STREET, LAKEFIELD ONTARIO

TOWNSHIP OF SELWYN

**DRAWING:** 

**SCHEDULES &** TYPICAL DETAILS





BARRY BRYAN ASSOCIATES
Architects Engineers Project Managers
250 Water Street Suite 201 Whitby Ontario L1N 0G5
T   (00T) 000 TOTO

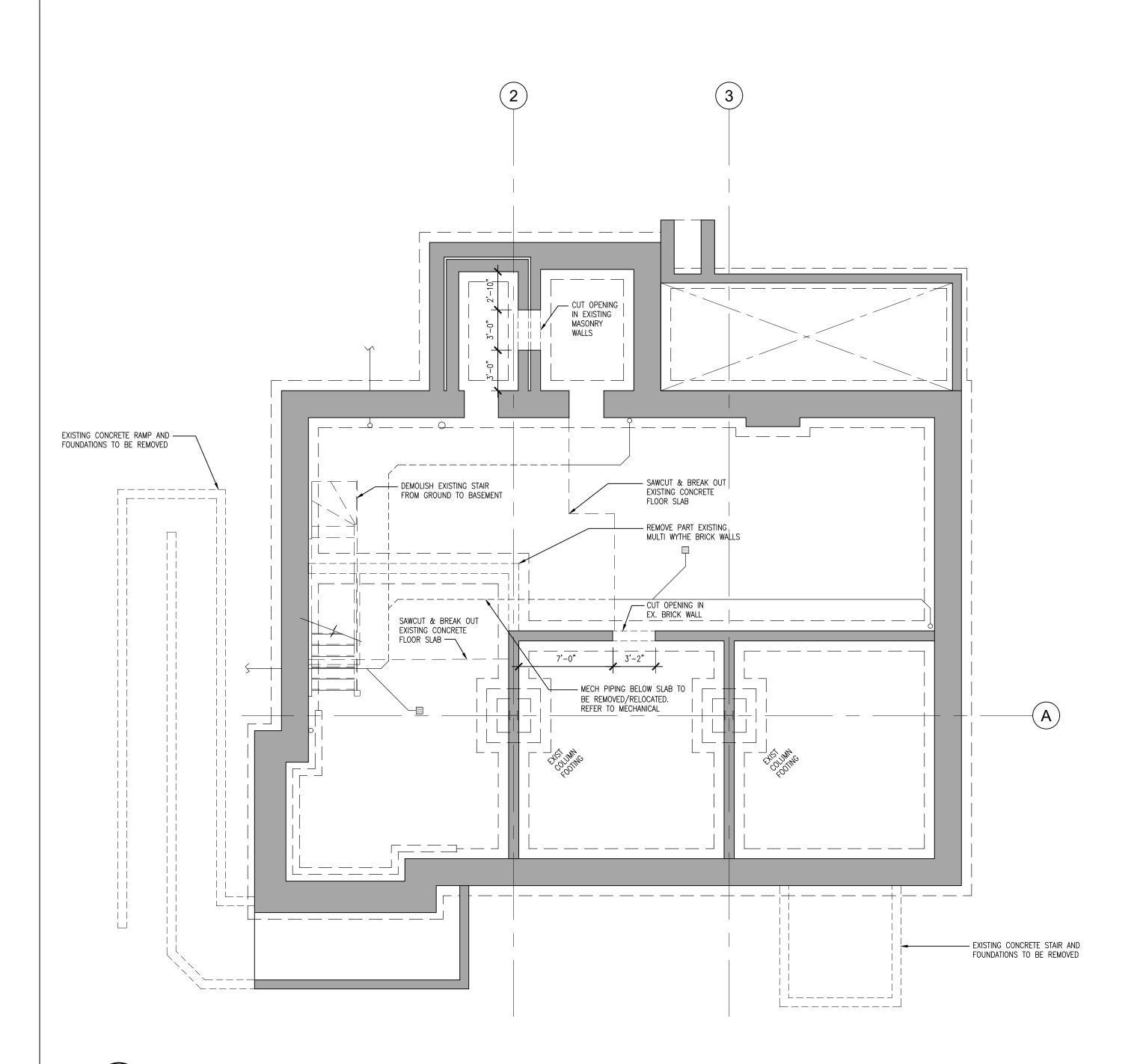
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BBA

PROJECT NO: 19152

DRAWING NO: **S102** 

% COMPLETE:



FOUNDATION PLAN - EXISTING

S201

MAKE GOOD FLOOR SLAB TO MATCH EXISTING --/ Hystall néw / LINTELS & MAKE GOOD FLOOR I SLAB TO MATCH /EXISTING/ \_\_\_\_\_EDGE OF NEW SLAB - PROVIDE 10M DOWELS 1.6" /LG @ /24" /o/c/EXPOXY /8" \ INTO EXISTING SLAB & NEW 4" SLAB ON GRADE TO MATCH EXISTING WF1/W1 INFILL WITH NEW 4" SOG \_\_REINF\_W/\_152\_x\_152\_x\_MW\_18.7\_x\_MW\_18.7\_WWM 8" CONC SLAB R/W 15M @ 10" BEW — /\_\_\_INSTALL NEW LINTEL & MAKE GOOD FLOOR SLAB /TØ/MATCH/EXISTING// ✓ A" HØUSEKEEPING – 12" SLAB ON GRADE. /R/W/15M/@/12"/BEW/ PROVIDE 10M DOWELS 16"-LG @ 24" o/c EXPOXY 8" INTO EXISTING SLAB & INFILL WITH NEW 4" SOG DRILL & EPOXY NEW DOWELS -16" LG @ 12" INTO EXISTING FOUNDATION WALL w/ 8" EMBED. FOR NEW FOUNDATION WALL SUPPORT. NEW CONCRETE WALL AND -FOOTING. REFER TO SECTION FOR DETAILS.

SEPARATE PRICE ITEM

IS INCLUDED IN TENDER PRICE

COMPLETELY BREAK OUT AND REMOVE EXISTING CONCRETE SLAB FOR EXTENT SHOWN HATCHED. EXCAVATE SUBGRADE AND REPLACE WITH MINIMUM 8" CRUSHED STONE FILL. INSTALL NEW 4" THICK CONCRETE SLAB THICK OVER ENTIRE AREA REINFORCED WITH 152 x 152 x MW 18.7 x MW 18.7 WELDED WIRE MESH. ALLOW FOR TOTAL 1290 SQUARE FEET OF NEW SLAB IN SEPARATE PRICE. AREA NOTED ABOVE DOES NOT INCLUDE PART OF CONCRETE SLAB SHOWN TO BE REMOVED AND REPLACED ON FOUNDATION PLANS, WHICH

FOUNDATION PLAN

S201

- 1. FINISHED GROUND FLOOR IS AT GEODETIC ELEVATION 00.00 EXCEPT AS CROSSED AND NOTED. ELEVATIONS FOR AREAS
- 2. ALL FOOTINGS SHALL BE CENTERED UNDER PIERS, WALLS AND/OR COLUMNS EXCEPT AS NOTED ON PLAN.
- 3. SEE ARCHITECTURAL DRAWINGS FOR SLOPED AND DRAINS. MAINTAIN SLAB THICKNESS SHOWN.
- 5. VERIFY EXISTING FOOTING ELEVATIONS AND SITE SERVICES PRIOR TO PROCEEDING WITH WORK. NOTIFY CONSULTANTS OF ANY DISCREPANCIES. REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
- 6. LOWER FOOTINGS TO ACCOMMODATE NEW AND/OR EXISTING FOOTINGS, MECHANICAL AND ELECTRICAL SERVICES. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR ELEVATIONS OF SAME. FOOTINGS ARE NOT TO BE UNDERMINED BY EXCAVATION FOR SERVICES, PITS, ETC.
- 7. PROTECT ALL EXISTING SUB GRADE SERVICES DURING INSTALLATION OF FOUNDATIONS.
- 8. REFER TO GENERAL NOTES AND TYPICAL DETAILS FOR ADDITIONAL INFORMATION.
- 9. PROVIDE DOWELS IN THE FOOTINGS TO MATCH ALL VERTICAL WALL REINFORCEMENT,
- 10. SDF DENOTES STEP DOWN FOOTINGS. REFER TO TYPICAL DETAILS.
- 11. REMOVE ALL TOPSOIL, LOOSE AND WET SOILS AND ORGANICS TO SUITABLE NATIVE SUB-GRADE MATERIAL AS APPROVED BY GEOTECHNICAL ENGINEER. THE EXISTING SUB-GRADE MUST BE PROOF ROLLED AND COMPACTED PRIOR TO

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### **Accessibility Renovations** - former Lakefield Post Office

12 QUEEN STREET, LAKEFIELD ONTARIO

TOWNSHIP OF SELWYN

DRAWING:

# FOUNDATION PLANS





% COMPLETE:

BARRY BRYAN ASSOCIATES
Architects Engineers Project Managers

BBA DRAWN BY: KM/CE CHECKED BY: DM 250 Water Street MAY 2022

Whitby Ontario L1N 0G5 Tel: (905) 666-5252 Fax: (905) 666-5256 e-mail: bba@bba-archeng.com

Suite 201

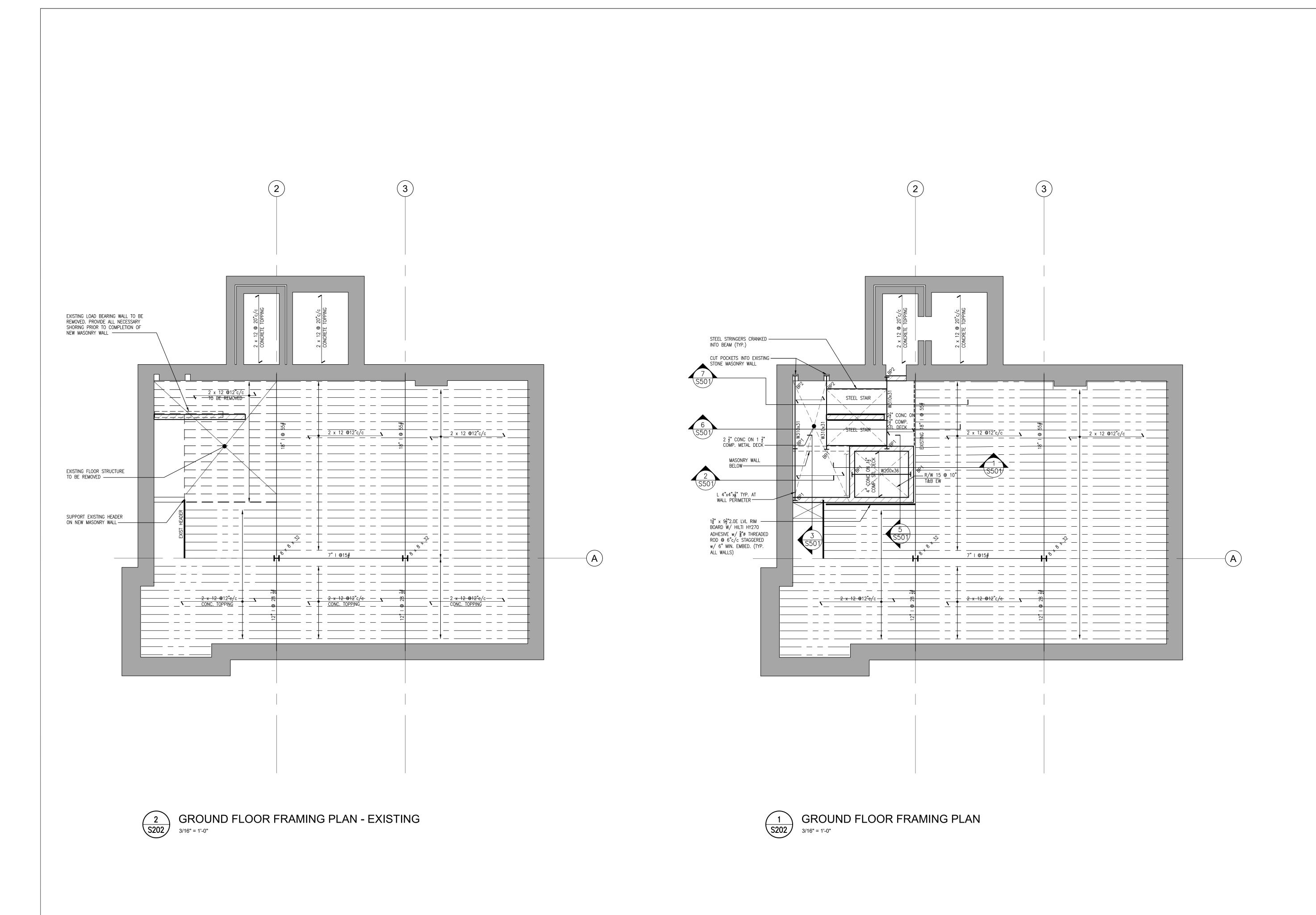
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PROJECT NO: 19152 **S201** 

FOUNDATION PLAN NOTES

- CROSSED AND NOTED ARE TO BE READ FROM THE FINISHED FLOOR ELEVATION UNLESS OTHERWISE NOTED.
- 4. ALL FOOTINGS SHALL EXTENDED TO OR BELOW SOUND BEARING STRATUM AS APPROVED BY GEOTECHNICAL ENGINEER.
  REFER TO PLAN FOR ULS AND BEARING CAPACITIES. GEOTECHNICAL ENGINEER SHALL APPROVE BEARING CONDITIONS ON SITE AND CONTRACTOR MUST REPORT ANY DOUBTFUL BEARING CONDITIONS.

- BACKFILLING.



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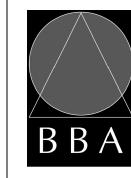
## **Accessibility Renovations** - former Lakefield Post Office

12 QUEEN STREET, LAKEFIELD ONTARIO

TOWNSHIP OF SELWYN

DRAWING:

**GROUND FLOOR** FRAMING PLANS





BARRY BRYAN ASSOCIATES Architects Engineers Project Managers 250 Water Street Suite 201

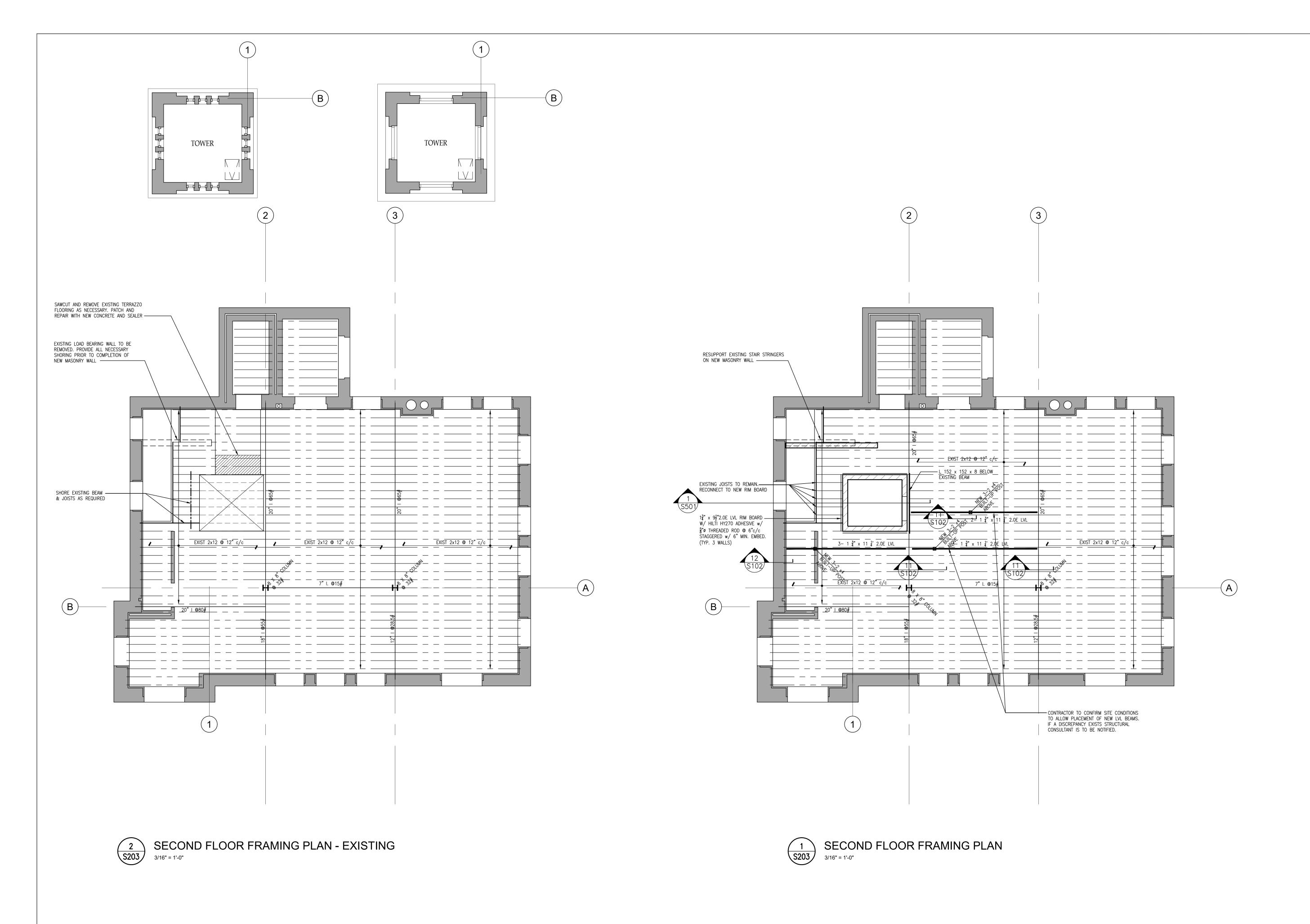
DESIGN BY: BBA DRAWN BY: KM/CE CHECKED BY: DM DATE: MAY 2022 Whitby Ontario L1N 0G5 Tel: (905) 666-5252

3/16"=1'-0"

Fax: (905) 666-5256 e-mail: bba@bba-archeng.com

**S202** 

19152



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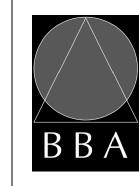
### **Accessibility Renovations** - former Lakefield Post Office

12 QUEEN STREET, LAKEFIELD ONTARIO

TOWNSHIP OF SELWYN

DRAWING:

SECOND FLOOR FRAMING PLANS



July 15, 2022
D. P. MCLAUGHLIN (100181883)
TO VINCE OF ONTARIO

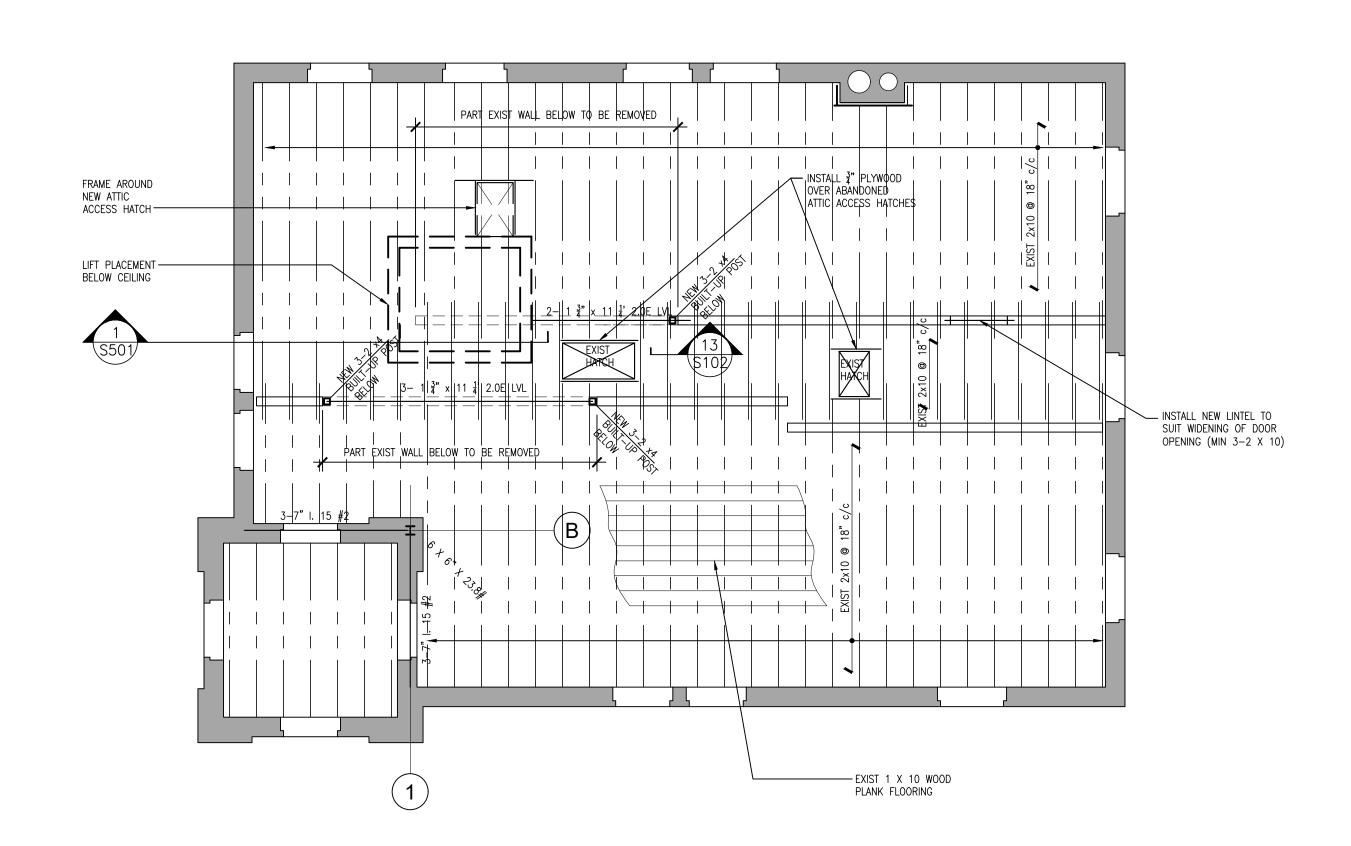
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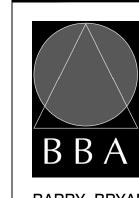
# Accessibility Renovations - former Lakefield Post Office

12 QUEEN STREET, LAKEFIELD ONTARIO

TOWNSHIP OF SELWYN

DRAWING:

ATTIC FRAMING PLAN



July 15, 2022
D. P. MCLAUGHLIN TO 100181883
HOUNCE OF ONTARE

BARRY BRYA ASSOCIATES
Architects Engineers Project Managers
250 Water Street

Engineers
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250 Water Street
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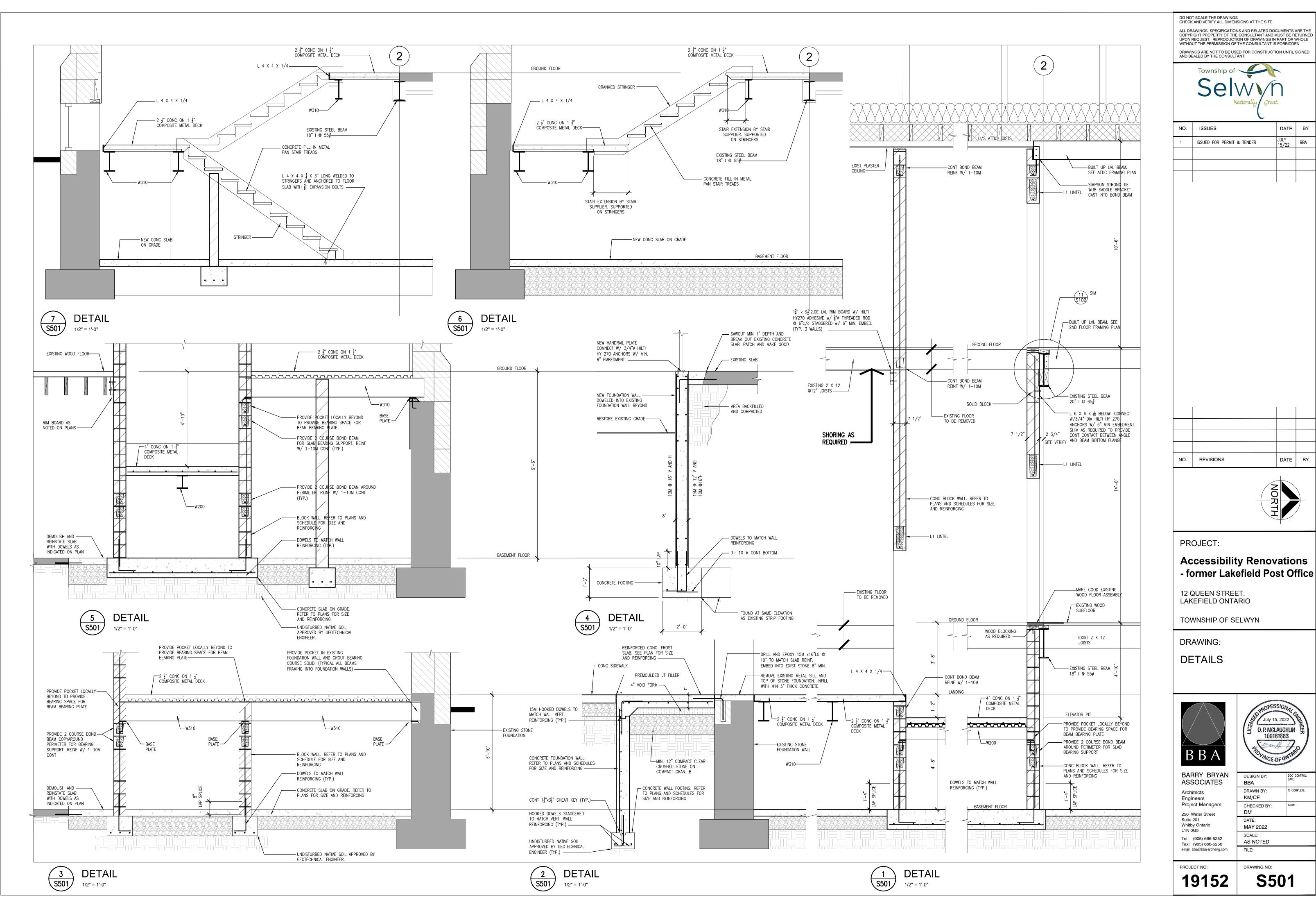
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