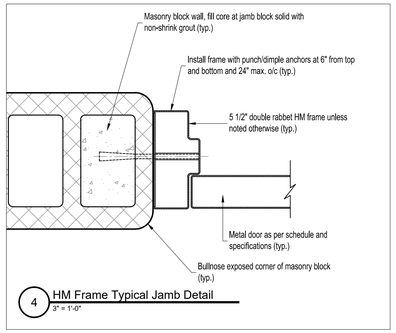


4 HM Frame Typical Jamb Detail
3" = 1'-0"

Building Code Analysis

Item	Ontario Building Code Matrix Parts 3 & 9					OBC Reference
1	Project Description:	☒ New ☐ Addition ☐ Change of Use ☐ Alteration		☐ Part 11	☒ Part 3	
2	Major Occupancy(s)	Group F-3				3.1.2.1.(1)
3	Building Area	Existing 0	New 65	Total 65		1.4.1.2.
4	Gross Area	Existing 0	New 65	Total 65		1.4.1.2.
5	Number of Storeys	Above Grade 1	Below Grade 0			3.2.1.1 & 1.4.1.2.
6	Number of Streets / Access Routes	2				3.2.2.10. & 3.2.5.5
7	Building Classification	Group F, Division 3, up to 2 Storeys				3.2.2.78
8	Sprinkler System Proposed	☐ entire building ☐ basement only ☐ in lieu of roof rating ☒ not required				3.2.2.20-.83 3.2.1.5 Index
9	Standpipe Required	☐ Yes ☒ No				3.2.9
10	Fire Alarm Required	☐ Yes ☒ No				3.2.4
11	Water Service / Supply Adequate	☒ Yes ☐ No				3.2.5.7
12	High Building	☐ Yes ☒ No				3.2.6
13	Construction Restrictions	☐ Combustible permitted ☐ Non-combustible required ☒ Both				3.2.2.28
	Actual Construction	☒ Combustible ☐ Non-combustible ☐ Both				
14	Mezzanine(s) Area	0.00 m²				3.2.6
15	Occupant load based on	☒ m²/ person ☐ design of building Total 1 persons				3.1.17.1.(1)(c)
16	Barrier-free	☒ Yes ☐ No (Explain)				3.8
17	Hazardous Substances	☐ Yes ☒ No				3.3.1.2. & 3.3.1.19.
18	Required Fire Resistance Rating (FRR)	Horizontal Assemblies FRR (Hours)		Listed Design No. or Description (SG-2)		3.2.2.20-.83 & 3.2.1.4
		Floors N/a Hours		N/a		
		Roof N/a Hours		N/a		
		Mezzanine N/a Hours		N/a		
19	Spatial Separation - Construction of Exterior Walls					3.2.3.

Wall	Area of EBF (m²)	L.D. (m)	L/H or H/L	Permitted Max. % of openings	Proposed % of Openings	FRR (Hours)	Listed Design or Description	Comb. Const.	Comb. Constr. Nonc. Cladding	Non-comb. Constr.
North	64.1	>10	<1:10	100	n/a	n/a	n/a	n/a	n/a	n/a
South	64.1	>10	>1:10	100	n/a	n/a	n/a	n/a	n/a	n/a
East	24.2	>8	<1:3	100	n/a	n/a	n/a	n/a	n/a	n/a
West	24.2	>8	<1:3	100	n/a	n/a	n/a	n/a	n/a	n/a

20	Required Plumbing Fixtures					
	Male/Female Count 50 %/ 50 %	Occupant Load	BC Table Number	Fixtures Required	Fixtures Provided	
	First Floor: Occupancy Both	< 10	N/A	1	4	3.7.4.9 (2)

Finish Hardware Specification

Continuous Hinges: ANSI A21011B Grade 1, 14 ga. (0.075" = 1.9mm) Type 204 Stainless Steel, US32D Satin Finish. 1/2" (6.3mm) diameter Stainless Steel Pin, Dual Ø Nylon bearings between each knuckle, door edge protection lip (both sides). Hager 790-905 or approved equivalent.
Locks & Latchsets: Heavy Duty mortise Schlage 'L' series with 06B lever/rose design with full size interchangeable core or approved equivalent.
Storeroom Lock: L9080R
Passage Set: L9010

Deadbolt: Heavy Duty Grade 1 Commercial Double Cylinder Deadbolt Schlage B662P with full size interchangeable core or approved equivalent.

Electric Strike: HES 1006 Series Heavy Duty Strike with Latchbolt Monitor - 630 Satin Stainless steel finish, FAIL SECURE or approved equivalent.

Door Operator: Hunter HA-8 Low Energy Operator with 2-6" round 10PBRLL push button actuator or approved equivalent.

Restroom Doors Control Kit: CX-WC11 by Camden Door Controls Ltd. or approved equivalent.

Restroom Emergency Call System: CX-WEC10 by Camden Door Controls Ltd. or approved equivalent.

Door Closer: Heavy Duty. Full rack and pinion hydraulic action. Cast iron cylinder body. Adjustible back check. Full plastic cover. DEL - delayed action, EDA - Extra duty arm. L.C.N. 4011 and 4111 series - no alternates.

Overhead Stop: Glynn Johnson surfae 90 series - no alternatives.

Weatherstripping: Extruded aluminum c/w neoprene insert. Predrill with oblong screw holes for adjustment. Designed to provide continuous weather seal. K.N. Crowder W-20N or equivalent.

Threshold: Extruded aluminum. Width to match door frame. Oversize in length to allow coping around exterior pressed steel frame faces. Expandable threshold to include insert(s) as required to suit frame jamb/sill detailing. K.N. Crowder CT series or equivalent.

Door Sweep: Extruded aluminum extrusion c/w black nylon bristles. K.N. Crowder W-24S or equivalent.

Door Control System

Provide door control system with remote access capability to control power supply to electric strikes and power door operators.

Note: All electric strikes are to be FAIL SECURE, hardware is formatted to allow emergency egress at all times.

Sequence of operations is as follows:

UNLOCKED
Power ON to electric strike and operator. Power operator functional upon activation by push button. Power supply to operator and electric strike interrupted by activation of the lock button (Interior Lock Button turns red when locked, exterior indicator illuminated to read "occupied").

LOCKED
Power OFF to electric strike and operator. Power operator will NOT function, electric strike locked in FAIL SECURE mode. All doors will function manually from interior (free egress) and with key from exterior.

Electric strikes are equipped with latchbolt monitor, system to indicate door latch position as OPEN or CLOSED.

Two (2) Universal Washrooms are equipped with Emergency Call feature, system to indicate emergency as EMERGENCY.

Door Hardware Sets

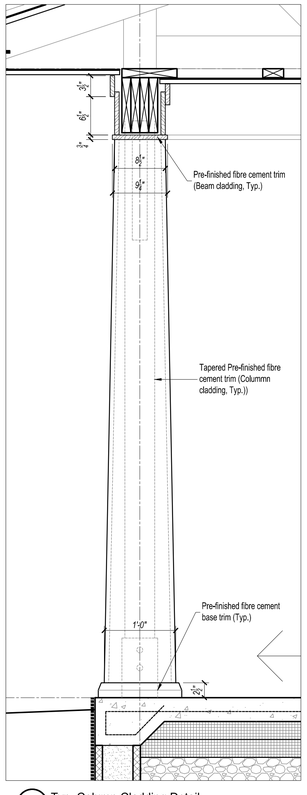
Set 01
1 - Continuous Hinge
1 - Door Closer
1 - Electric Strike
1 - Storeroom Lock
1 - Door Operator
2 - Door Actuator
1 - Door Lock Button
Weatherstripping
Barrier Free Threshold
Door sweep

Set 02
1 - Continuous Hinge
1 - Passage Set
1 - Deadbolt

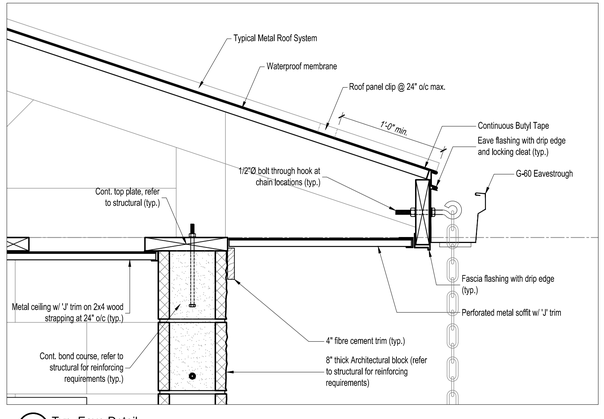
Set 03
1 - Continuous Hinge
1 - Storeroom Lock
1 - Overhead Stop
Weatherstripping
Barrier Free Threshold
Door sweep

DOOR AND FRAME SCHEDULE

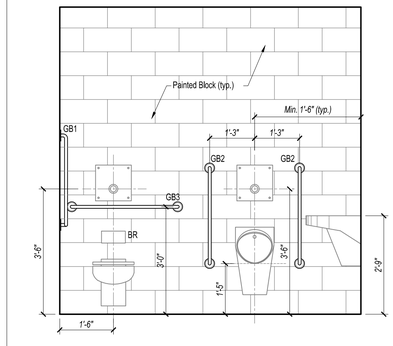
MARK	DOOR SIZE WD	SIZE HGT	SIZE THK	DOOR MATL	FRAME MATL	FRAME DETAIL HEAD	FRAME DETAIL JAMB	HARDWARE SET NO	HARDWARE KEYSIDE RM NO	NOTES
1	3'-0"	7'-0"	1 3/4"	HM	HM	–	–	01	EXT.	–
2	3'-0"	7'-0"	1 3/4"	HM	HM	–	–	02	BOTH	–
3	3'-0"	7'-0"	1 3/4"	HM	HM	–	–	01	EXT.	–
4	3'-0"	7'-0"	1 3/4"	HM	HM	–	–	02	BOTH	–
5	3'-0"	7'-0"	1 3/4"	HM	HM	–	–	01	EXT.	–
6	3'-0"	7'-0"	1 3/4"	HM	HM	–	–	02	BOTH	–
7	3'-0"	7'-0"	1 3/4"	HM	HM	–	–	01	EXT.	–
8	3'-0"	7'-0"	1 3/4"	HM	HM	–	–	02	BOTH	–
9	3'-0"	7'-0"	1 3/4"	HM	HM	–	–	03	EXT.	–

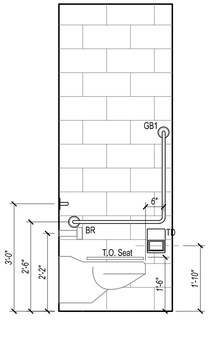


5 Typ. Column Cladding Detail
1" = 1'-0"



3 Typ. Eave Detail
1 1/2" = 1'-0"





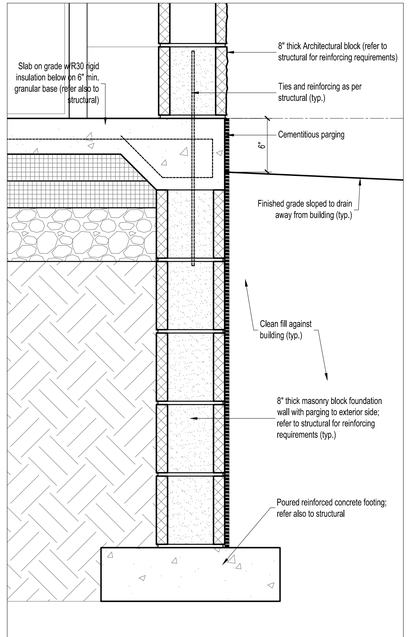
1 Typ. Washroom Elevations
1/2" = 1'-0"

Ontario Building Code References

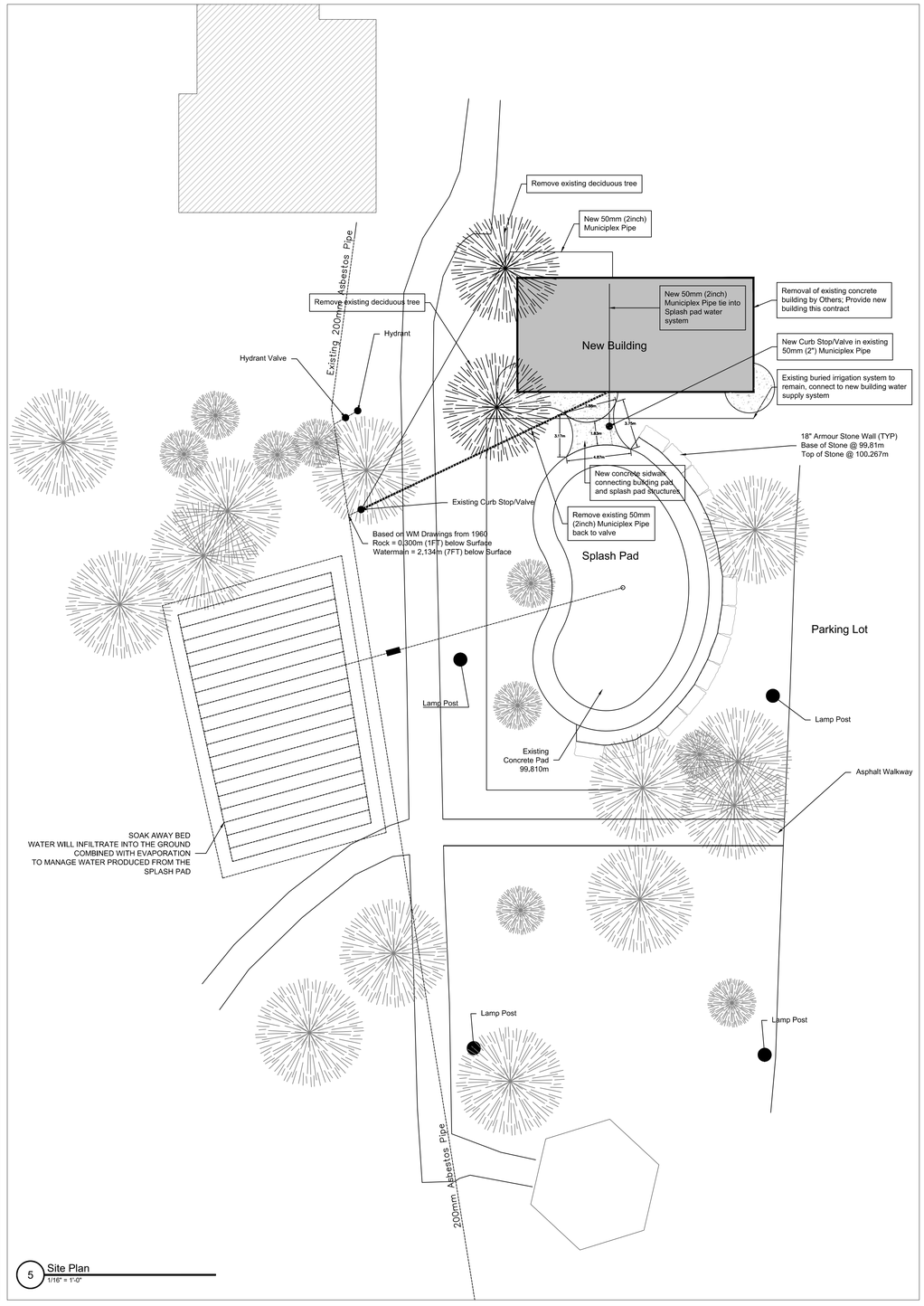
3.8.3. Design Standards
3.8.3.1. Accessibility Signs
3.8.3.2. Exterior Walkways
3.8.3.3. Doorways & Doors
3.8.3.8. Water Closet Stalls
3.8.3.9. Water Closets
3.8.3.10. Urinals
3.8.3.11 Lavatories
3.8.3.12 Universal Washrooms

The contractor must be conversant and complete all installations in compliance with these requirements.

The contractor shall provide four (4) coat hooks and a 6"x12" long shelf in each washroom. Location to be determined on site.



2 Typ. Foundation Detail
1 1/2" = 1'-0"



5 Site Plan
1/16" = 1'-0"

RONALD AWDE ARCHITECT
1458 ▪ KING ▪ STREET
BETHANY ▪ ONTARIO
705▪277▪9490

NOTES

1. DRAWINGS AND SPECIFICATIONS ARE PROVIDED AS AN INSTRUMENT OF SERVICE AND REMAIN THE PROPERTY OF RONALD A. AWDE ARCHITECT. ALL COPYRIGHT CONDITIONS ARE RESERVED BY THE ARCHITECT WITH RESPECT TO THESE DOCUMENTS. THESE DOCUMENTS SHALL NOT BE DUPLICATED OR USED FOR OTHER THAN THE PURPOSE FOR WHICH THEY WERE ISSUED.

2. THE CONTRACTOR WILL VERIFY ALL DIMENSIONS FOR THE WORK AND SHALL REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO COMMENCEMENT OF WORK.

3. DRAWINGS ARE NOT TO BE SCALED FOR CONSTRUCTION PURPOSES.

4. THE ARCHITECT IS NOT RESPONSIBLE FOR THE ACCURACY OF SURVEY, STRUCTURAL, MECHANICAL, ELECTRICAL, ETC. INFORMATION SHOWN ON THIS DRAWING. REFER TO THE APPROPRIATE CONSULTANT'S DRAWINGS BEFORE PROCEEDING WITH THE WORK.

5. CONSTRUCTION MUST CONFORM TO ALL APPLICABLE CODES AND REQUIREMENTS OF AUTHORITIES HAVING JURISDICTION.

6. THE CONTRACTOR WORKING FROM DRAWINGS NOT SPECIFICALLY MARKED 'FOR CONSTRUCTION' MUST ASSUME FULL RESPONSIBILITY AND BEAR COSTS FOR ANY CORRECTIONS OR DAMAGES RESULTING FROM THIS WORK.

7. NO CHANGES SHALL BE MADE TO THE WORK DESCRIBED IN THESE DRAWINGS OR SPECIFICATIONS WITHOUT THE EXPRESS WRITTEN AUTHORIZATION OF THE ARCHITECT.

PROJECT

Garnet G. Graham Park Pavilion

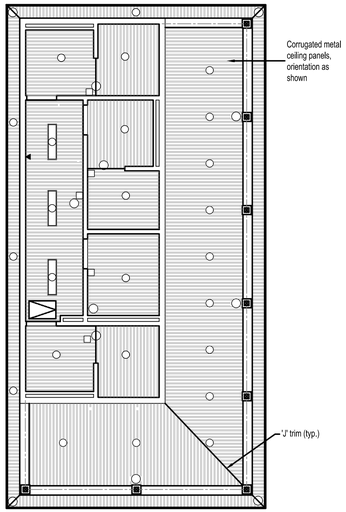
MARK	DATE	ISSUE
4.0	2020.09.24	Issued for Tender
3.1	2020.08.28	ADO Button Location
3.0	2020.05.26	Issued For Permit
2.0	2018-05-29	90% Review
1.0	2018-02-08	Conceptual Design Review

PROJECT NO: 2018-02
DRAWN BY: C.C.
CHK'D BY: R.A.

SHEET TITLE
Site Plan & Details

DRAWING NO.
A-01

RONALD
AWDE
ARCHITECT
1458 ▪ KING ▪ STREET
BETHANY ▪ ONTARIO
705▪277▪9490

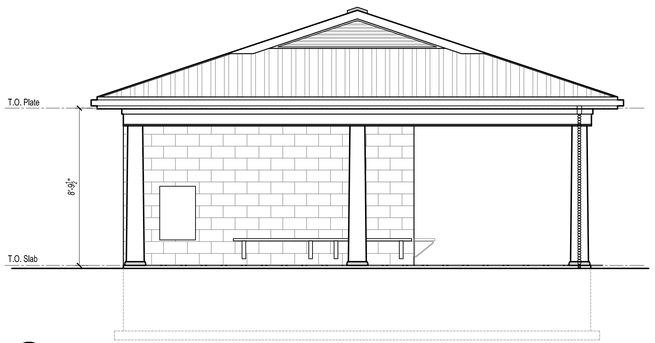


7 Reflected Ceiling Plan
1/8" = 1'-0"

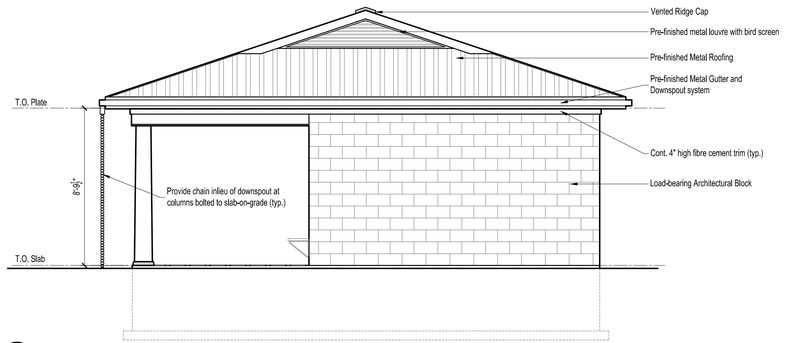
Drawing Legend

O/S	Occupancy Sensor (Lights)
CCTV	Security Camera (Security)
EA	Emergency Alert Button
DL	Door Lock Button (Door Hardware)
ES	Electric Strike (Door Hardware)
PB	Automatic Operator Button (Door Hardware)
	Abuse Resistant LED Strip Light
	Abuse Resistant LED Light Fixture
EL	Emergency LED Lighting

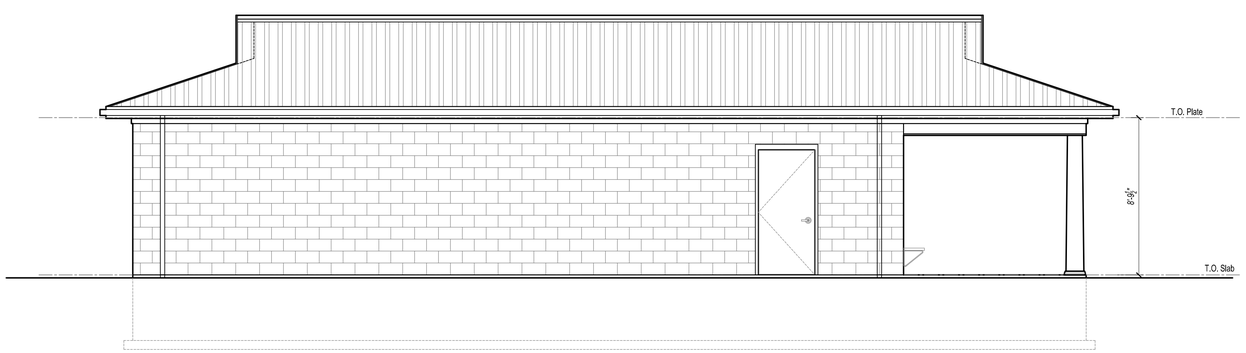
6 South Elevation
1/4" = 1'-0"



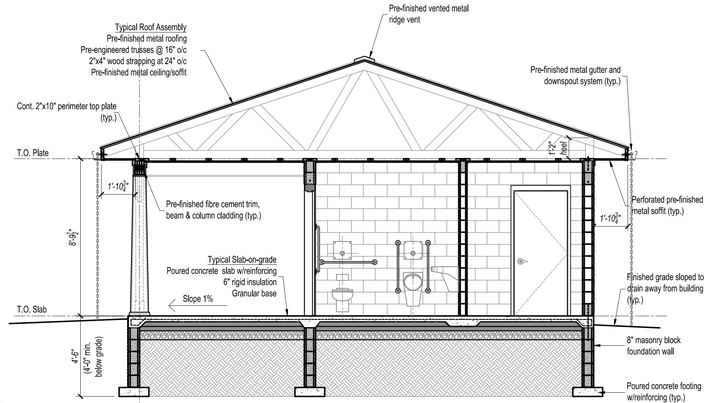
5 West Elevation
1/4" = 1'-0"



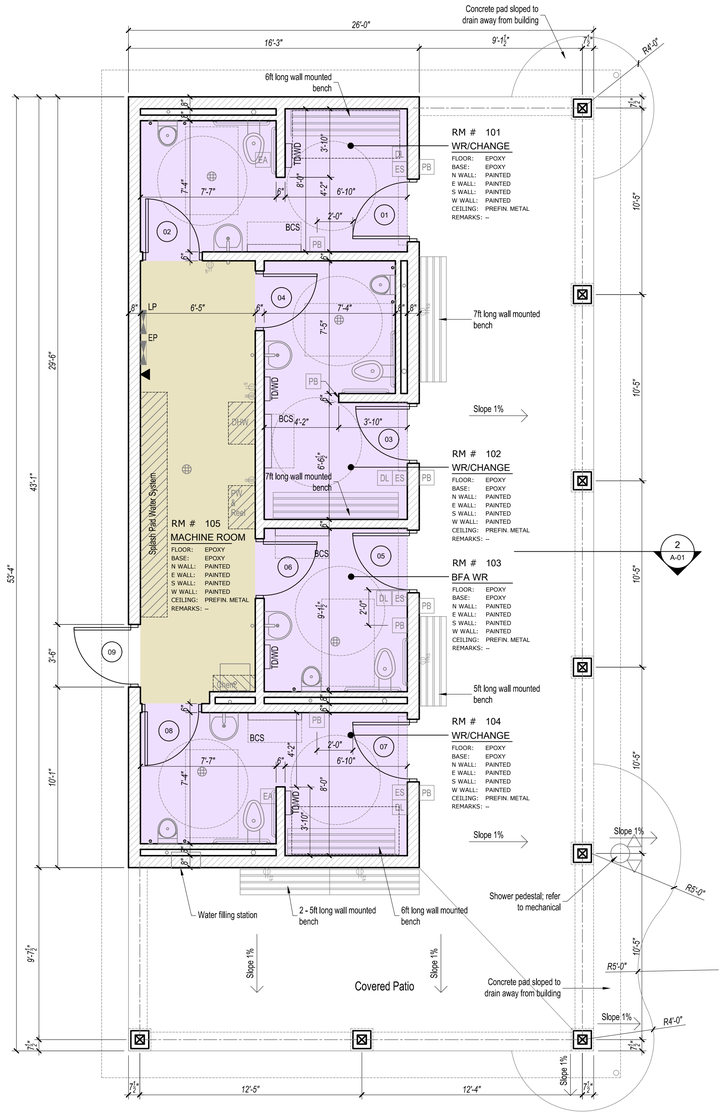
4 East Elevation
1/4" = 1'-0"



3 North Elevation
1/4" = 1'-0"



2 Building Section
1/4" = 1'-0"



NOTES

1. DRAWINGS AND SPECIFICATIONS ARE PROVIDED AS AN INSTRUMENT OF SERVICE AND REMAIN THE PROPERTY OF RONALD A. AWDE ARCHITECT. ALL COPYRIGHT CONDITIONS ARE RESERVED BY THE ARCHITECT WITH RESPECT TO THESE DOCUMENTS. THESE DOCUMENTS SHALL NOT BE DUPLICATED OR USED FOR OTHER THAN THE PURPOSE FOR WHICH THEY WERE ISSUED.
2. THE CONTRACTOR WILL VERIFY ALL DIMENSIONS FOR THE WORK AND SHALL REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO COMMENCEMENT OF WORK.
3. DRAWINGS ARE NOT TO BE SCALED FOR CONSTRUCTION PURPOSES.
4. THE ARCHITECT IS NOT RESPONSIBLE FOR THE ACCURACY OF SURVEY, STRUCTURAL, MECHANICAL, ELECTRICAL, ETC. INFORMATION SHOWN ON THIS DRAWING. REFER TO THE APPROPRIATE CONSULTANT'S DRAWINGS BEFORE PROCEEDING WITH THE WORK.
5. CONSTRUCTION MUST CONFORM TO ALL APPLICABLE CODES AND REQUIREMENTS OF AUTHORITIES HAVING JURISDICTION.
6. THE CONTRACTOR WORKING FROM DRAWINGS NOT SPECIFICALLY MARKED "FOR CONSTRUCTION" MUST ASSUME FULL RESPONSIBILITY AND BEAR COSTS FOR ANY CORRECTIONS OR DAMAGES RESULTING FROM THIS WORK.
7. NO CHANGES SHALL BE MADE TO THE WORK DESCRIBED IN THESE DRAWINGS OR SPECIFICATIONS WITHOUT THE EXPRESS WRITTEN AUTHORIZATION OF THE ARCHITECT.

PROJECT

Garnet G. Graham Park Pavilion

MARK	DATE	ISSUE
4.0	2020.09.24	Issued for Tender
3.1	2020.08.28	ADO Button Location
3.0	2020.05.26	Issued For Permit
2.0	2018-05-29	90% Review
1.0	2018-02-08	Conceptual Design Review

PROJECT NO:	2018-02
DRAWN BY:	C.C.
CHK'D BY:	R.A.

SHEET TITLE
Plan, Elevations and Section

DRAWING NO.
A-02

GENERAL NOTES

1. THESE DRAWINGS ARE TO BE READ IN CONJUNCTION WITH THE STRUCTURAL SPECIFICATIONS AND ALL OTHER CONTRACT DRAWINGS AND THEIR RESPECTIVE SPECIFICATIONS.
2. ALL DIMENSIONS ARE IN INCHES EXCEPT AS NOTED. THE CONTRACTOR SHALL REVIEW ALL CONTRACT DOCUMENTS PRIOR TO AND DURING CONSTRUCTION. WHERE THE CONTRACTOR FINDS AN ERROR, INCONSISTENCY, OR OMISSION RELATING TO THE CONTRACT, THE CONTRACTOR SHALL PROMPTLY REPORT IT TO THE ENGINEER AND SHALL NOT PROCEED WITH THE ACTIVITY AFFECTED UNTIL RECEIVING DIRECTION FROM THE ENGINEER.
3. THE DRAWINGS SHOW THE COMPLETED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR SAFETY ON THE JOB SITE AND FOR DESIGN, INSTALLATION AND SUPERVISION OF ALL TEMPORARY BRACING AND FALSEWORK TO SUIT HIS CONSTRUCTION METHODS AND TO SUPPORT THE SUPERIMPOSED CONSTRUCTION LOADS.
4. ALL WORK AND MATERIALS SHALL CONFORM TO REQUIREMENTS SET OUT IN THE 2012 ONTARIO BUILDING CODE.
5. ALL WORK IS TO BE CARRIED OUT IN ACCORDANCE WITH THE OCCUPATIONAL HEALTH AND SAFETY ACT OF ONTARIO.
6. SUB-BASE UNDER SLABS ON GRADE SHALL BE COMPACTED TO 98% STANDARD PROCTOR DENSITY UNLESS NOTED OTHERWISE. COMPACTION SHALL BE VERIFIED IN WRITING BY THE SOILS ENGINEER PRIOR TO POURING OF SLABS.
7. BACKFILLING AGAINST WALLS ON ONE SIDE ONLY SHALL NOT BE STARTED UNTIL TEMPORARY BRACING OR FLOOR SLABS PROVIDING SUPPORT ARE IN PLACE AND SET.
8. SOIL BEARING CAPACITY SHALL BE VERIFIED BY THE SOILS ENGINEER IN WRITING PRIOR TO POURING OF FOOTINGS. FOUNDATIONS HAVE BEEN DESIGNED FOR AN ASSUMED ALLOWABLE SOIL BEARING CAPACITY OF 75 kPa. THE OWNER IS RESPONSIBLE FOR HAVING THE BEARING CAPACITY VERIFIED.
9. ALL DESIGN LOADS NOTED ON DRAWINGS ARE WORKING LOADS (UNLESS NOTED OTHERWISE).
10. DESIGN LOADS:
    a) DEAD LOAD: = 20 psf
    b) SNOW LOAD: = 47 psf (CASE 1), 54 psf (CASE 2)
    c) WIND LOAD: = 7.5 psf (q1/50)
    d) EARTHQUAKE LOAD = AS PER O.B.C. SB-1 FOR FENELON FALLS
11. SHOP DRAWINGS SHALL BE SUBMITTED WHERE REQUESTED FOR SPECIFIC STRUCTURAL ELEMENTS. SUBMISSIONS SHALL BE REVIEWED AND ACCEPTED BY THE ENGINEER OF RECORD PRIOR TO FABRICATION. DIGITAL DRAWING FILES OF THE STRUCTURAL DRAWINGS WILL NOT BE MADE AVAILABLE TO THE CONTRACTOR FOR THE PURPOSE OF PREPARING SHOP DRAWINGS. THE FOLLOWING SHOP DRAWINGS SHALL BE SUBMITTED:
    a) CONCRETE MIX DESIGNS
    b) REINFORCING STEEL.
    c) PRE-FABRICATED ROOF TRUSSES (P.ENG. STAMP REQUIRED)
12. ALL CODES AND STANDARDS REFERENCED SHALL BE THE LATEST EDITION REFERENCED BY THE 2012 ONTARIO BUILDING CODE (DIV.B, 1.3.1.2).

CONCRETE

1. ALL CONCRETE SHALL CONFORM TO CAN/CSA-A23.1. AND BE READY MIX.
2. CONCRETE SHALL BE PROPORTIONED IN ACCORDANCE WITH CAN/CSA A23.1 AS FOLLOWS:

CONCRETE	EXPOSURE CLASS	MIN. 28 DAY COMPRESSIVE STRENGTH (MPa)	AIR CONTENT CATEGORY
1) FOOTINGS	C-4	25	2
2) FOUNDATION WALLS, PIERS, INTERIOR SLABS ON GRADE	F-2	25	2
3) EXTERIOR SLABS ON GRADE	C-1	35	1

   - 75% TYPE 'GU' PORTLAND CEMENT +25% SLAG CEMENT
   - 20mm MAXIMUM NOMINAL SIZE OF COURSE AGGREGATE
3. ALL CONCRETE ADDITIVES SHALL BE APPROVED BY THE ENGINEER.
4. NO CONCRETE SHALL BE POURED WITHOUT PRIOR REVIEW OF THE ENGINEER.
5. ALL CONCRETE SHALL BE TESTED IN ACCORDANCE WITH CAN/CSA-A23.2.
6. FOR COMPRESSIVE STRENGTH TESTING OF CONCRETE A MINIMUM OF 3-150x600 CYLINDERS ARE REQUIRED:
    A) EACH TYPE/GRADE OF CONCRETE
    B) FORMED EACH CHANGE OF SUPPLIER
    C) EACH 20 CU m OR FRACTION THEREOF FOR COLUMNS AND SHEAR WALLS
    D) EACH 50 CU m OR FRACTION THEREOF FOR FOOTINGS, FOUNDATION WALLS, SLABS AND BEAMS
    E) ADDITIONAL TEST SPECIMENS SHALL BE TAKEN WHENEVER REQUESTED BY THE ENGINEER OR THE SUPERVISOR TO VERIFY THE CONCRETE QUALITY
8. ALL MIX DESIGNS SHALL CONFORM TO CAN/CSA-A23.1.
9. ALL CONCRETE SHALL BE WET CURED FOR A MINIMUM OF 7 DAYS AFTER PLACEMENT OF CONCRETE. CURING SHALL CONFORM TO CAN/CSA-A23.1.
10. CONCRETE PROTECTIVE COVER FOR REINFORCING STEEL SHALL BE AS FOLLOWS:
    A) EXPOSED TO FILL - U/S OF FOOTING 3"
    B) FORMED AND AGAINST FILL 2"
    C) WALLS & SLABS EXPOSED TO WATER 2"
    D) WALLS NOT EXPOSED TO WATER 1"
    E) SLABS NOT EXPOSED TO WATER 1"
11. ALL EXPOSED CORNERS OF CONCRETE SURFACES TO HAVE ¾" x ¾" CHAMFER.

REINFORCING STEEL

1. ALL REINFORCING STEEL SHALL HAVE A MINIMUM YIELD STRENGTH OF 400 MPa AND SHALL CONFORM TO CAN/CSA G30.18.
2. ALL REINFORCING STEEL SHALL BE DETAILED, FABRICATED, PLACED AND SUPPORTED IN ACCORDANCE WITH REINFORCING STEEL MANUAL OF STANDARD PRACTICE BY THE REINFORCING STEEL INSTITUTE OF CANADA AND CAN/CSA-A23.3.
3. ALL WWF SHALL CONFORM TO CAN/CSA G30.3 AND CAN/CSA G30.5. ALL WWF SHALL BE SUPPLIED IN FLAT SHEETS ONLY. INSTALL AS PER DETAILS ON THIS DRAWING.
4. REINFORCING BARS SHALL BE LAPPED AS FOLLOWS WHEN SPLICED:
   - 15M 25"
   - 20M 33"
5. THE CONTRACTOR SHALL PROVIDE CONTINUOUS SUPERVISION DURING PLACEMENT OF CONCRETE TO ENSURE THAT THE REINFORCING STEEL IS MAINTAINED IN ITS CORRECT POSITION.
6. REINFORCING STEEL THAT IS STORED ON SITE IS TO BE PLACED ON BLOCKS SO THAT IT IS NOT IN DIRECT CONTACT WITHT THE GROUND AND SHALL BE PROTECTED FROM RAIN AND SNOW WITH THE USE OF TARPAULINS.
7. ELECTRICAL CONDUITS AND PLUMBING PIPING TO BE PLACED IN THE CENTRE OF WALLS OR SLABS. LOCATION OF ANY CONDUIT WITH OUTSIDE DIAMETER EQUAL TO OR GREATER THAN 25% OF SLAB THICKNESS TO BE APPROVED BY ENGINEER.

FORMWORK

1. DESIGN, CONSTRUCT AND REMOVE FORMWORK, FRAMING SUPPORTS AND BRACING TO CONFORM TO REQUIREMENTS SPECIFIED IN CAN/CSA-A23.1, AND CAN/CSA S269.1, TO PROVIDE FINISHED POURED CONCRETE SURFACES WITHIN SPECIFIED TOLERANCES.
2. ALLOWABLE TOLERANCES TO REQUIREMENTS OF CAN/CSA-A23.1.
3. CHAMFER ALL EXTERNAL CORNERS EXPOSED TO VIEW.
4. INSTALL ITEMS SUPPLIED BY OTHERS SUCH AS INSERTS, ANCHOR BOLTS, MISCELLANEOUS FRAMES, METAL FLASHING REGLETS, HOLES, SLEEVES, LADDER RUNGS AND DOVETAIL ANCHOR SLOTS.
5. DO NOT REMOVE FORMS OR SHORES, WITHOUT PRIOR APPROVAL OF THE ENGINEER.
6. FORMS SHALL NOT BE REMOVED BEFORE THE CONCRETE HAS SET AND REACHED 70% OF ITS DESIGN STRENGTH:
    A) BEAM SOFFITS 7 DAYS
       RESHORE 28 DAYS
    B) SLABS 7 DAYS
       RESHORE 28 DAYS
    C) SIDES OF BEAMS, COLUMNS AND WALLS 3 DAYS
7. CONSTRUCTION JOINTS SHALL BE LOCATED SO AS TO LEAST IMPAIR THE STRENGTH OF THE STRUCTURE AND TO THE ENGINEER'S APPROVAL. CONSTRUCTION JOINTS SHALL BE KEYED AND 15M DOWELS X 1050mm LONG AT 600mm o.c. SHALL BE ADDED. REINFORCING SHALL NOT BE INTERRUPTED.
8. REMOVE ALL FINS FROM VISIBLE SURFACES. FILL ALL THE HOLES WITH PLASTIC PLUGS, CAULKING AND GROUT.

WOOD FRAMING

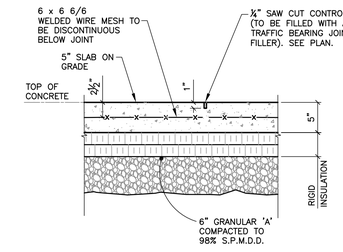
1. ALL COMPONENTS OF ALL WOOD ROOF TRUSSES SHALL BE DESIGNED, FABRICATED AND INSTALLED IN ACCORDANCE WITH CAN/CSA-086 AND THE ONTARIO BUILDING CODE (OBC) PART 4.
2. ROOF TRUSS LOADINGS:
    A) SNOW LOADS

Case 1 SNOW LOAD = 47 psf

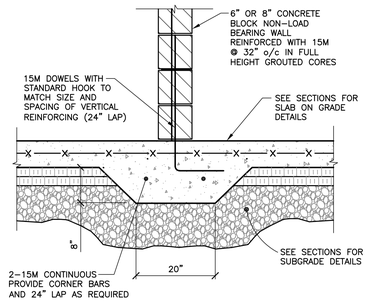
Case 2 SNOW LOAD = 54 psf

    B) SUPERIMPOSED DEAD LOAD
       TOP CHORD = 8 psf
       BOTTOM CHORD = 7 psf
    C) WIND UPLIFT
       WIND UPLIFT = AS PER O.B.C. REQUIREMENTS
       IMPORTANCE CATEGORY = NORMAL

    NOTE: LOADS DO NOT INCLUDE THE FOLLOWING:
    i) WEIGHT OF ROOF TRUSSES
    ii) DRIFTING SNOW IN VALLEYS
    iii) WEIGHT OF JACK TRUSSES OR COMMON FRAMING BUILT ON TOP OF MAIN TRUSSES
3. ALL TRUSS SHOP DRAWINGS TO BE SUBMITTED FOR REVIEW BY THE ENGINEER PRIOR TO FABRICATION AND SHALL HAVE THE STAMP OF A PROFESSIONAL ENGINEER LICENSED IN ONTARIO. DRAWINGS ARE TO INCLUDE HANGERS, CONNECTIONS AND DETAILS OF LATERAL BRACING.
4. LIMIT ROOF TRUSS LIVE LOAD DEFLECTION TO L/360th OF SPAN.
5. PROVIDE TWO UPLIFT CONNECTORS AT EACH TRUSS BEARING POINT (H2.5T BY SIMPSON STRONG-TIE).
6. AT TRUSS BEARING POINTS WHERE THE ALLOWABLE COMPRESSION PERPENDICULAR TO THE GRAIN IS EXCEEDED, THE TRUSS SUPPLIER SHALL PROVIDE BEARING PLATES.
7. ALL LUMBER SHALL BE No.1/2 GRADE SPF IN ACCORDANCE WITH CAN/CSA-086 UNLESS NOTED OTHERWISE. ALL ROOF SHEATHING TO BE A MINIMUM OF ⅝" T&G PLYWOOD, UNLESS NOTED OTHERWISE.
8. ALL NAILS, SPIKES AND STAPLES SHALL BE IN ACCORDANCE WITH OBC CLAUSE 9.23.3.
9. TEMPORARY LATERAL BRACING DURING FLOOR AND ROOF TRUSS ERECTION AND PRIOR TO ROOF SHEATHING AND ATTACHMENT OF CEILING FINISH IS THE RESPONSIBILITY OF THE TRADE CONTRACTOR.
10. PRE-DRILL ALL LAG BOLT HOLES PRIOR TO INSTALLING BOLTS.
11. PROVIDE SEALED SHOP DRAWINGS FOR ALL SPECIALTY HANGERS.
12. ALL LVL LUMBER SHALL BE 2800-Fb 2.0E MATERIAL AND SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS AND REQUIREMENTS.
13. LVL PLIES IN BUILT UP BEAMS SHALL BE FASTENED TO ADJACENT PLIES WITH 4 ROWS OF 3¼" LONG COMMON NAILS AT 12" o.c. OR IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.
14. SPF PLIES IN BUILT UP BEAMS SHALL BE FASTENED TO ADJACENT PLIES WITH 4-3" LONG COMMON NAILS AT 12" o.c.
15. SPECIFIED CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE AND INSTALLED IN ACCORDANCE WITH MANUFACTURER'S WRITTEN SPECIFICATIONS. SUBSTITUTIONS SHALL BE SUBMITTED FOR ENGINEER'S APPROVAL PRIOR TO CONSTRUCTION.
16. ALL WOOD SCREWS SHALL MEET THE REQUIREMENTS OF ASME B18.6.1.



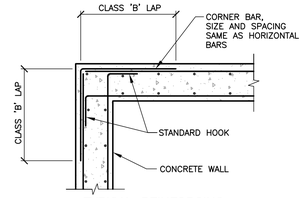
TYPICAL CONTROL JOINT
1"=1'-0"



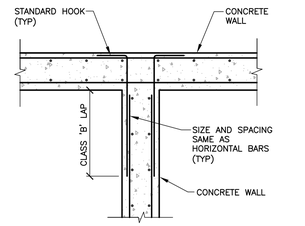
SLAB ON GRADE UNDER NON-LOAD BEARING
CONCRETE BLOCK WALL PARTITION
¾"=1'-0"

MASONRY

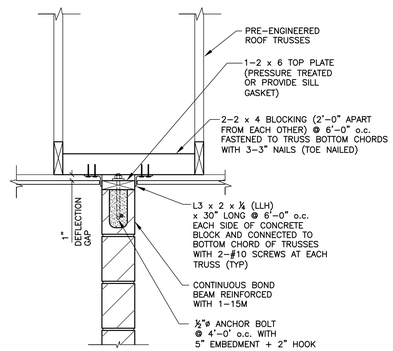
1. ALL MASONRY CONSTRUCTION TO CSA-A371 MASONRY CONSTRUCTION FOR BUILDINGS.
2. ALL MASONRY UNITS TO BE GRADE 'A' UNITS TO CSA-A165, UNLESS NOTED OTHERWISE.
3. ALL MASONRY UNITS TO CSA-A371.
4. ALL MASONRY MORTAR FOR BLOCKS SHALL BE TYPE 'S' TO CSA-A179.
5. ALL MASONRY MORTAR FOR BRICK VENEER SHALL BE TYPE 'N' TO CSA-A179.
6. ALL GROUT TO HAVE MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 15 MPa AND BE IN ACCORDANCE WITH CSA A179.
7. MASONRY UNITS:
   -METHOD B
   -CSA-A165 AND A179.
   -15MPa COMPRESSIVE STRENGTH (BASED ON NET AREA)
8. MASONRY CONSTRUCTION
   -RUNNING BOND
   -MASONRY BONDING OF CORNERS
   -FULL HEAD & BED JOINTS
9. TESTING
   -3 MASONRY UNITS PER 5000 sq.ft. OF WALL
   -MORTAR TO CSA-A179 FOR 465 sq.ft. OF WALL
   -GROUT TO CSA-A179 FOR 530 cu.ft.
10. REINFORCING:
    -JOINT 200MPa ASTM A82 AND CSA G30.3
    -BARS 400MPa CSA G30.18
11. ALL MASONRY UNITS SHALL BE PILED ABOVE GROUND IN DRY LOCATION. ALL MASONRY UNITS SHALL BE PROTECTED WITH TARPAULINS.
12. ALL MORTAR AND GROUT MATERIAL SHALL BE STORED UNDER COVER IN A DRY PLACE.
13. NO ADMIXTURES SHALL BE USED IN EITHER MORTAR OR GROUT WITHOUT THE ENGINEERS PRIOR APPROVAL.
14. GROUT FOR REINFORCED HOLLOW UNITS SHALL BE MIXED BY VOLUME AS FOLLOWS:
    A) 1-PART PORTLAND CEMENT
    B) 3-PARTS MASONRY SAND
    C) 3-PARTS PEA GRAVEL
    D) GROUT SHALL HAVE A SLUMP OF 10"-11"
15. ALL CONCRETE BLOCK WALLS SHALL BE REINFORCED WITH LADDER TYPE HORIZONTAL BLOCK REINFORCEMENT EVERY SECOND COURSE TO S304.1. LAP JOINT REINFORCEMENT 8" AT EACH SPLICE.
16. HOLLOW MASONRY UNITS SHALL BE LAID WITH FACE SHELL BED AND HEAD JOINTS. IN ADDITION, THE WEBS SHALL BE LAID IN A FULL BED IN ALL COURSES OF PIERS, COLUMNS AND PILASTERS AND IN THE STARTING COURSE ON FOOTINGS, SOLID FOUNDATION WALLS AND WHERE ADJACENT TO CELLS OR CAVITIES THAT ARE TO BE REINFORCED OR FILLED WITH GROUT OR CONCRETE.
17. ALL INTERSECTING OR ABUTTING MASONRY MUST BE BONDED BY MASONRY UNITS.
18. WHERE MASONRY VENEER IS BACKED UP BY CONCRETE WALL PROVIDE VERTICAL DOVETAIL ANCHOR SLOTS HORIZONTALLY SPACED AT 2'-0"o.c.
19. ALL MORTAR JOINTS SHALL BE ⅜".
20. GROUT SPACES SHALL NOT BE WETTED DOWN PRIOR TO POURING GROUT.
21. VERTICAL CELLS TO BE FILLED SHALL HAVE A VERTICAL ALLIGNMENT TO MAINTAIN A CONTINUOUS UNOBSTRUCTED CELL AREA OF NOT LESS THAN 2"x 3".
22. GROUT SHALL BE POURED IN MAX. LIFTS OF 4'-0" o.c. AT LEAST ONE (1) HOUR SHALL ELAPSE BETWEEN GROUTING SUCCESIVE LIFTS.
23. ALL GROUT SHALL BE RECONSOLIDATED ONE (1) HOUR AFTER PLACING BY INSERTING VIBRATOR 300mm INTO LOWER LIFT OF GROUT.
24. ALL GROUT SHALL BE CONSOLIDATED WITH A ¾ FLEXIBLE CABLE VIBRATOR.
25. ALL MASONRY PARTITIONS SHALL BE SUPPORTED ALONG THE TOP AS PER THE TYPICAL DETAILS. A DEFLECTION SPACE OF SPAN DIVIDED BY 240 SHALL BE ALLOWED AT THE TOP OF MASONRY PARTITIONS.
26. LINTELS FOR NONBEARING WALL SHALL BE AS FOLLOWS (UNLESS NOTED OTHERWISE ON PLANS):
    -200mm DEEP BLOCK LINTELS REINFORCED WITH 2-15M BOTTOM UP TO 1200mm SPAN.
    -2-20M BOTTOM UP TO 1800mm SPAN.
    -FILL WITH 15 MPa GROUT. MORTAR IS NOT PERMITTED.
27. TOP 2 COURSES OF BLOCK BEARING WALLS SUPPORTING FLOORS AND ROOF FRAMING MUST BE FILLED SOLID WITH GROUT.
28. FOR GROUTED MASONRY WALLS THE CONTRACTOR MAY USE EITHER LOW LIFT OR HIGH LIFT GROUTING METHOD IN ACCORDANCE WITH CSA-A179/CSA-A371. BEFORE PROCEEDING WITH WORK, OBTAIN THE ENGINEERS APPROVAL FOR ALL DETAILS OF THE PROPOSED PROCEDURE. GIVE SUFFICIENT NOTICE TO ALLOW INSPECTION OF WORK ON SITE.
29. MASONRY CONTRACTOR MUST EXERCISE UTMOST CARE TO PROVIDE FULL MORTAR JOINT COVERAGE ON ALL BEARING SURFACES OF MASONRY IN ORDER TO DEVELOP THE FULL BEARING CAPACITY OF STRUCTURAL MASONRY WALLS. ANY MASONRY FALLING SHORT OF THE ABOVE REQUIREMENTS MUST BE REPLACED.
30. STEEL ANGLES FOR SUPPORT OF MASONRY TO HAVE EVEN AND LEVEL BEARING NOT LESS THAN 8" UNLESS NOTED OTHERWISE.
31. NON-LOAD BEARING BLOCK PARTITIONS SHALL BE 6" WIDE MINIMUM AND SHALL BE REINFORCED WITH 15M FULL HEIGHT @ 32" o/c. REINFORCED CORES SHALL BE GROUTED SOLID. PROVIDE DOWELS IN ACCORDANCE WITH TYPICAL SLAB ON GRADE UNDER NON-LOAD BEARING CONCRETE BLOCK WALL PARTITION DETAIL ON S-1.



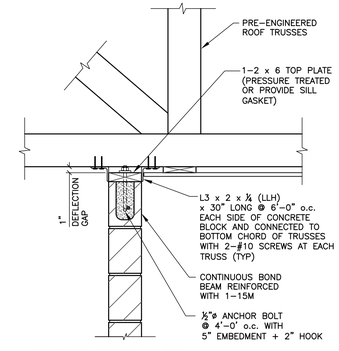
TYPICAL REINFORCING
WALL REINFORCING PLAN
½"=1'-0"



TYPICAL PLAN DETAIL
REINFORCING AT WALL INTERSECTION
½"=1'-0"



LATERAL SUPPORT FOR
NON-LOAD BEARING BLOCK WALLS
PARALLEL TO ROOF TRUSSES
1"=1'-0"



LATERAL SUPPORT FOR
NON-LOAD BEARING BLOCK WALLS
PERPENDICULAR TO ROOF TRUSSES
1"=1'-0"

DRAWINGS ARE NOT TO BE USED FOR CONSTRUCTION UNLESS SPECIFICALLY NOTED AS 'ISSUED FOR CONSTRUCTION' IN THE REVISION BLOCK.

No.	REVISION DESCRIPTION	DATE
1.	ISSUED FOR CLIENT REVIEW	APR.24/18
2.	ISSUED FOR CLIENT REVIEW	MAY 9/18
3.	ISSUED FOR PERMIT AND CONSTRUCTION	JUN.30/20

ENGINEER STAMP

LICENSED PROFESSIONAL ENGINEER
M.A. SANFILIPPO
100114494
PROVINCE OF ONTARIO

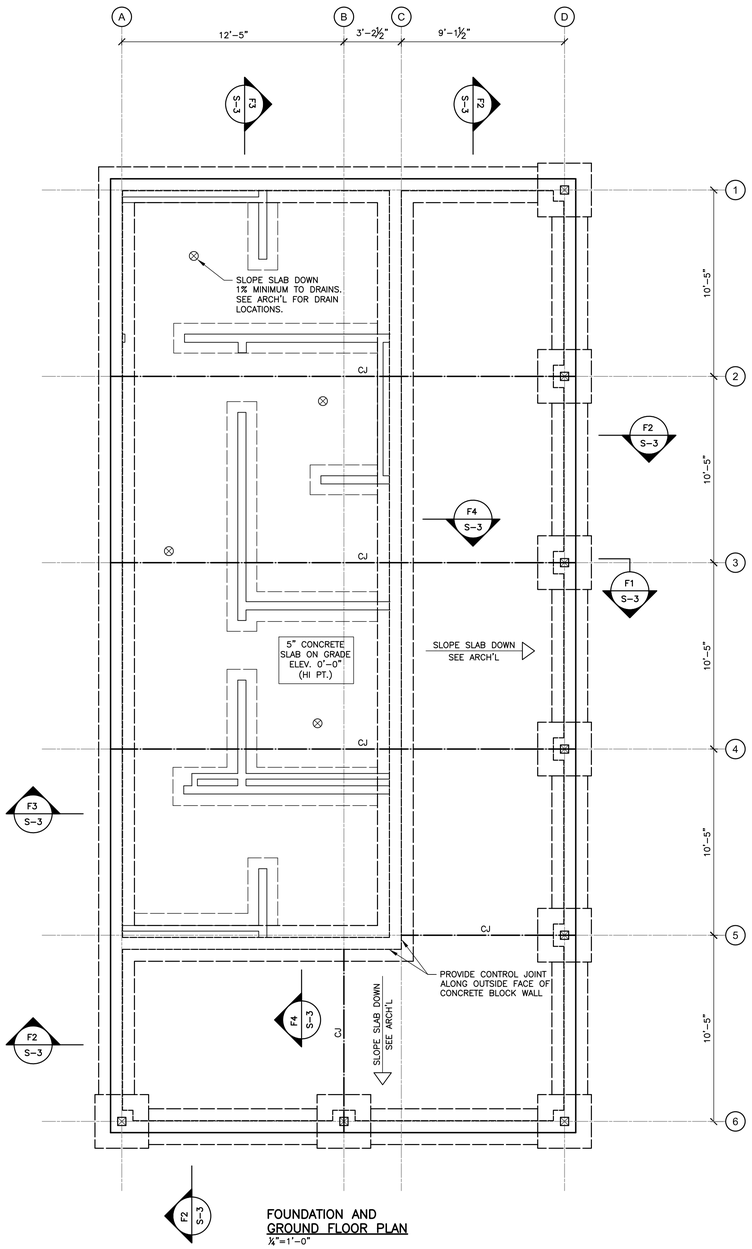
PARK PAVILION
GARNET GRAHAM PARK
CITY OF KAWARTHA LAKES, ON

GENERAL NOTES
AND
TYPICAL DETAILS

TATHAM ENGINEERING

DESIGN: MAS	FILE: 418359-1	DWG: S-1
DRAWN: WHG	DATE: APR 2018	
CHECK: JJ	SCALE: AS NOTED	

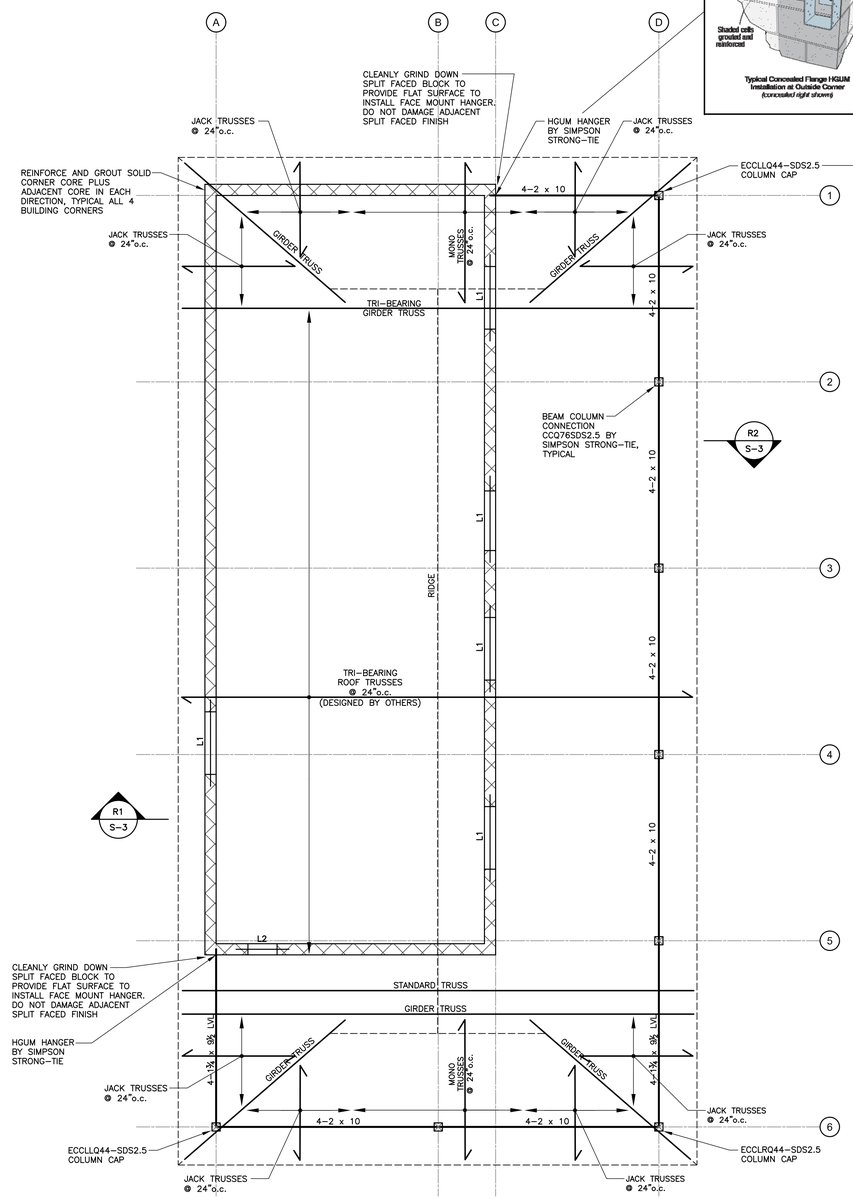
Drawing Name: 418359 - S001.dwg, Plotted: Jun 30, 2020

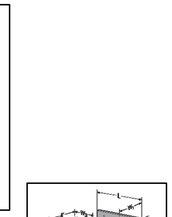


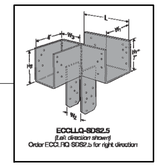
FOUNDATION AND GROUND FLOOR PLAN

¼"=1'-0"

1. 'CJ' DENOTES SAW CUT CONTROL JOINT IN CONCRETE SLAB (SEE TYPICAL DETAIL ON S-1).





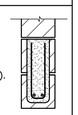


ROOF FRAMING PLAN

¼"=1'-0"

1. ALL CONNECTORS TO BE SIMPSON STRONG-TIE.

LINTEL SCHEDULE

L1	7½" WIDE x 16" DEEP BOND BEAM FILLED SOLID WITH 20 MPa GROUT c/w 2-15M CONTINUOUS BOTTOM BARS AND 10M @ 8" o.c. VERTICAL LEG STIRRUPS. PROVIDE MIN. 8" BEARING LENGTH EACH END ON SOLID FILLED MASONRY (2 COURSES DEEP). EXTEND BARS 6" PAST OPENING AT EACH END AND PROVIDE 2" HOOK EACH END.	
L2	2-L4 x 3½ x ¼" (LLV) WITH 8" MINIMUM BEARING LENGTH EACH END ON GROUTED MASONRY	

DRAWINGS ARE NOT TO BE USED FOR CONSTRUCTION UNLESS SPECIFICALLY NOTED AS 'ISSUED FOR CONSTRUCTION' IN THE REVISION BLOCK.

No.	REVISION DESCRIPTION	DATE
1.	ISSUED FOR CLIENT REVIEW	APR.24/18
2.	ISSUED FOR CLIENT REVIEW	MAY 9/18
3.	ISSUED FOR PERMIT AND CONSTRUCTION	JUN.30/20

ENGINEER STAMP

LICENSED PROFESSIONAL ENGINEER
M.A. SANFILIPPO
100114494
PROVINCE OF ONTARIO

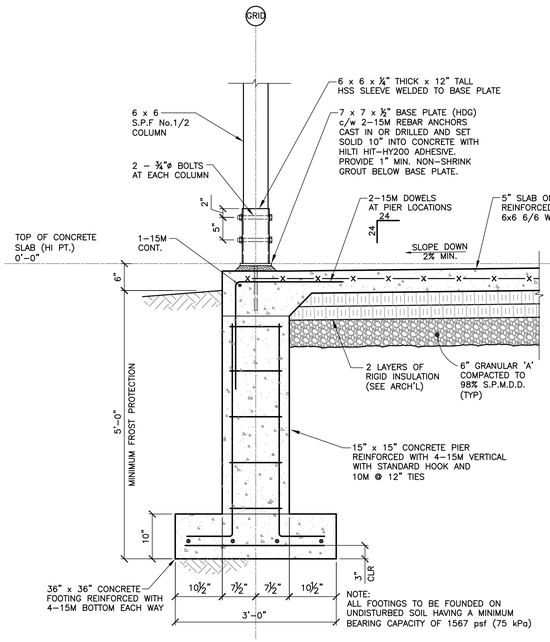
PARK PAVILION
GARNET GRAHAM PARK
CITY OF KAWARTHA LAKES, ON

PLANS

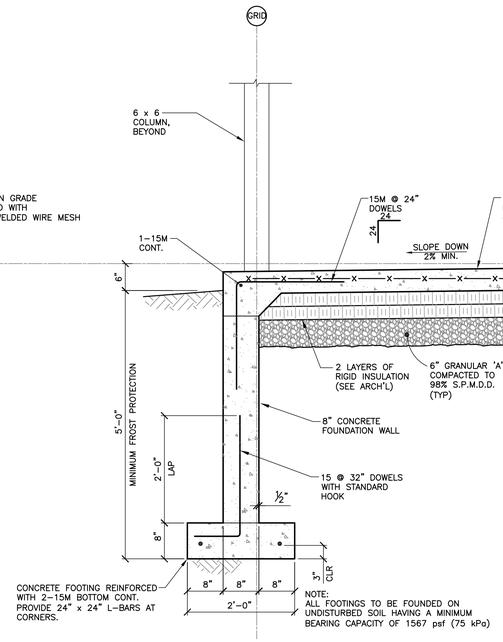
TATHAM ENGINEERING

DESIGN: MAS	FILE: 418359-1	DWG: S-2
DRAWN: WHG	DATE: APR 2018	
CHECK: JJ	SCALE: AS NOTED	

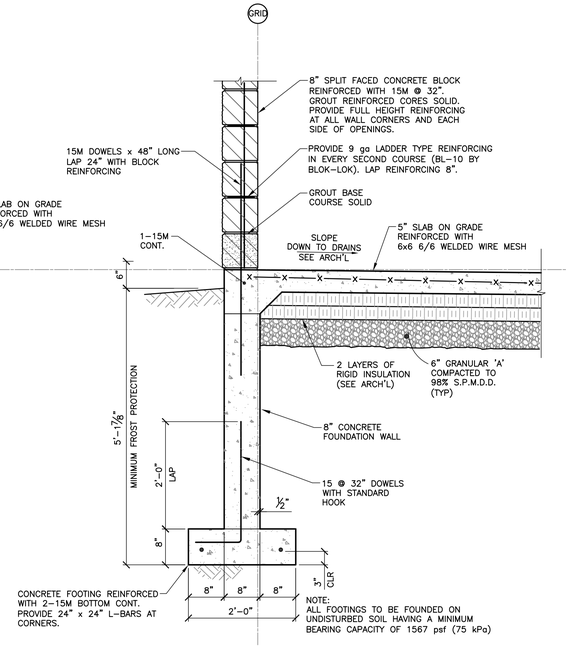
Drawing Name: 418359 – S001.dwg, Plotted: Jun 30, 2020



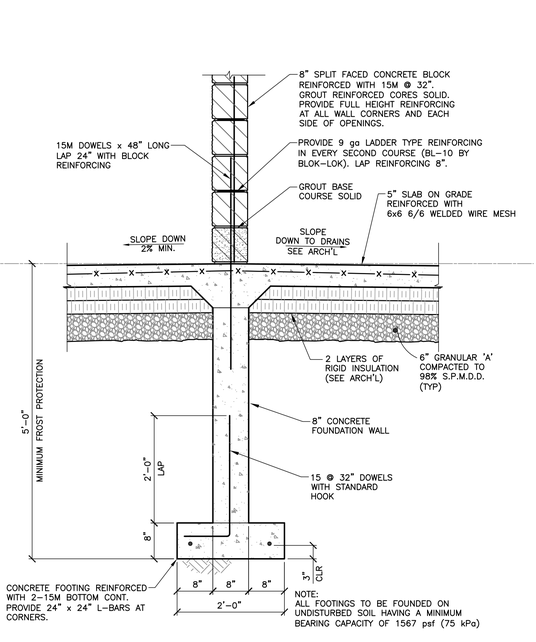
F1 SECTION S–2 ¾"=1'–0"



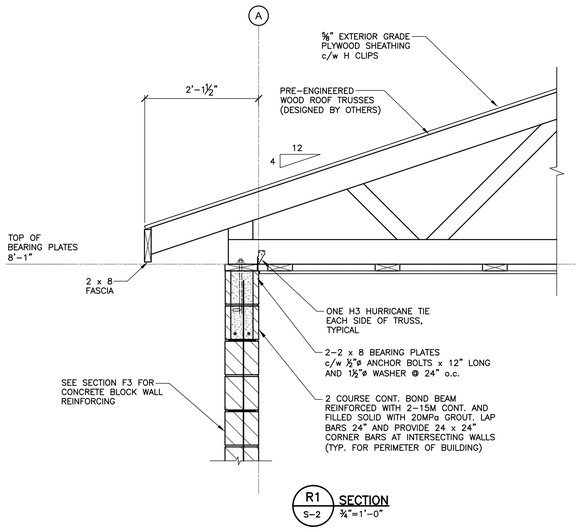
F2 SECTION S–2 ¾"=1'–0"



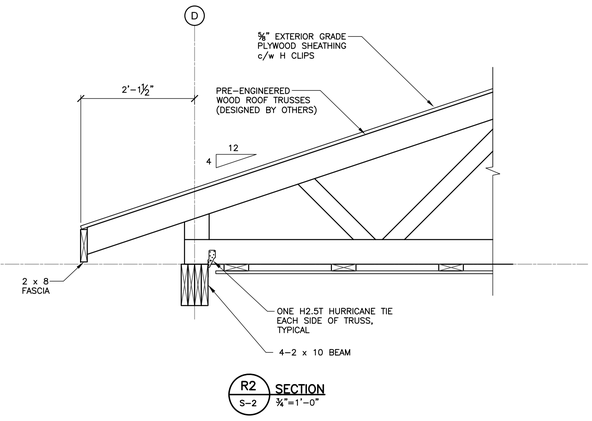
F3 SECTION S–2 ¾"=1'–0"



F4 SECTION S–2 ¾"=1'–0"



R1 SECTION S–2 ¾"=1'–0"



R2 SECTION S–2 ¾"=1'–0"

DISCLAIMER AND COPYRIGHT

CONTRACTOR MUST VERIFY ALL DIMENSIONS AND BE RESPONSIBLE FOR SAME. ANY DISCREPANCIES MUST BE REPORTED TO THE ENGINEER BEFORE COMMENCING WORK. DRAWINGS ARE NOT TO BE SCALED.

DRAWINGS ARE NOT TO BE USED FOR CONSTRUCTION UNLESS SPECIFICALLY NOTED AS 'ISSUED FOR CONSTRUCTION' IN THE REVISION BLOCK.

No.	REVISION DESCRIPTION	DATE
1.	ISSUED FOR CLIENT REVIEW	APR.24/18
2.	ISSUED FOR CLIENT REVIEW	MAY 9/18
3.	ISSUED FOR PERMIT AND CONSTRUCTION	JUN.30/20

ENGINEER STAMP

LICENSED PROFESSIONAL ENGINEER
M.A. SANFILIPPO
100114494
Jun. 30/20
PROVINCE OF ONTARIO

PARK PAVILION
GARNET GRAHAM PARK
CITY OF KAWARTHA LAKES, ON

TATHAM ENGINEERING

SECTIONS

DESIGN: MAS	FILE: 418359–1	DWG: S-3
DRAWN: WHG	DATE: APR 2018	
CHECK: JJ	SCALE: AS NOTED	

Drawing Name: 418359 – S001.dwg, Plotted: Jun 30, 2020